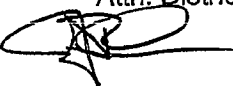




# Illinois Department of Transportation

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To: Omer Osman Attn: District Nine  
From: John D. Baranzelli   
Subject: Pavement Design  
Date: March 29, 2012

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FAP Route 869 (IL Route 34)  
Section 105R-1  
Saline County  
From Ellis Road to the Levee, north of Harrisburg

We have reviewed the pavement selection for the above captioned section, which was submitted with your memorandum dated March 15, 2012. Based on life cycle cost, the rigid design was the most economical choice for the calculated pavement designs. The district requested the alternate bid option for this project to receive competitive bids. BDE concurs with the alternate bid request. The approved pavement designs are as follows:

IL Route 34 (Pavement Reconstruction)

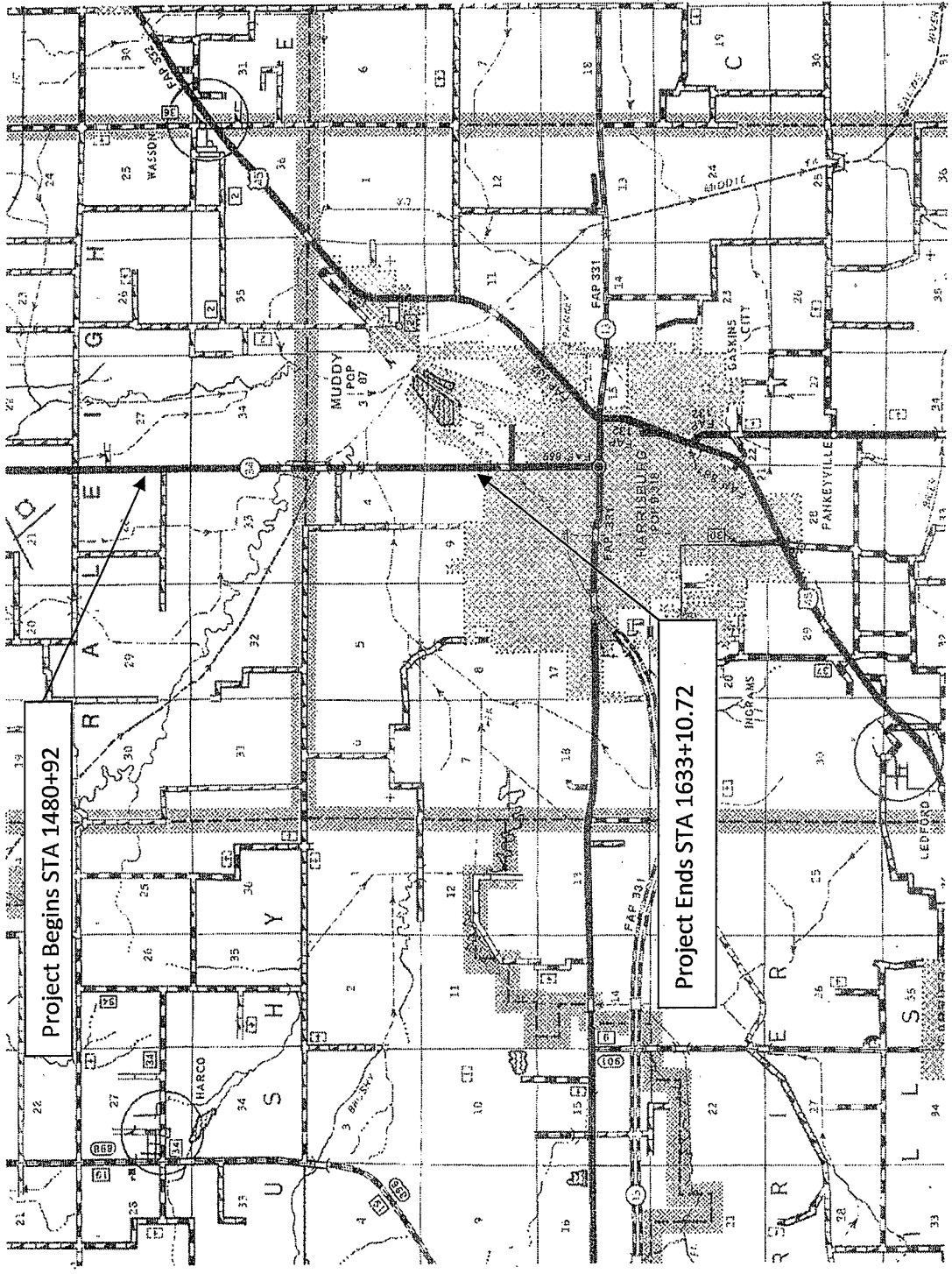
Option 1:

15.75 inches of HMA Pavement  
2 inches of Polymerized HMA Surface Course, Mix "D", N105  
2.25 inches of Polymerized HMA binder course, IL-19.0, N105  
11.5 inches of HMA Binder Course, IL-19.0, N90  
16 inches of Lime Stabilization

Option 2:

10.5 inches of Jointed PCC Pavement built 14' wide to eliminate  
the need for tied shoulders  
4 inches of Stabilized Sub-Base  
16 inches of Lime Stabilization

If you have any questions, please contact Paul Niedernhofer at (217) 524-1651.



Project Begins STA 1480+92

Project Ends STA 1633+10.72

PAVEMENT DESIGN

CALC CWS

ILL 34 CLASS II STREET DATE 2-22-12

SHEET 1 OF 4

		RAW DATA	RAW DATA	
EXIST	2011 ADT	6-29-11	% MINE	2012 (1.50%)
ADT	4300	4604	4504	5225
SU	215	214	214	248
MU	900	1057	57	66
PV	3185	3333	3333	3866

MINE OPERATOR ESTIMATES THEY WILL BE SENDING OUT 900 TRUCKS PER DAY BY JUNE 2012 - ASSUME THIS IS THE MAX CAPACITY FOR THE MINE - REDUCE RAW DATA MUs BY PRESENT MINE TRAFFIC OF 500 TRUCKS

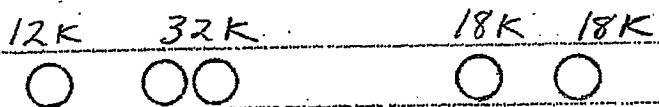
Flexible

$$\text{NORMAL TF} = 20 \left[ \frac{(0.15)(0.5)(3866) + (112.06)(0.5)(248) + (385.44)(0.5)(66)}{1,000,000} \right]$$

$$\text{NORMAL TF} = 0.53 \checkmark$$

MINE OPERATOR STATED TRUCKS WERE LOADED SO THAT THE TOTAL WEIGHT OF THE TRUCK IS 80,000 LBS.

ASSUME TYPICAL AXLE LOAD DISTRIBUTION IS THE SAME AS THE EXAMPLE



FLEXIBLE PAVEMENT ESAL VALUES (TABLE 1)

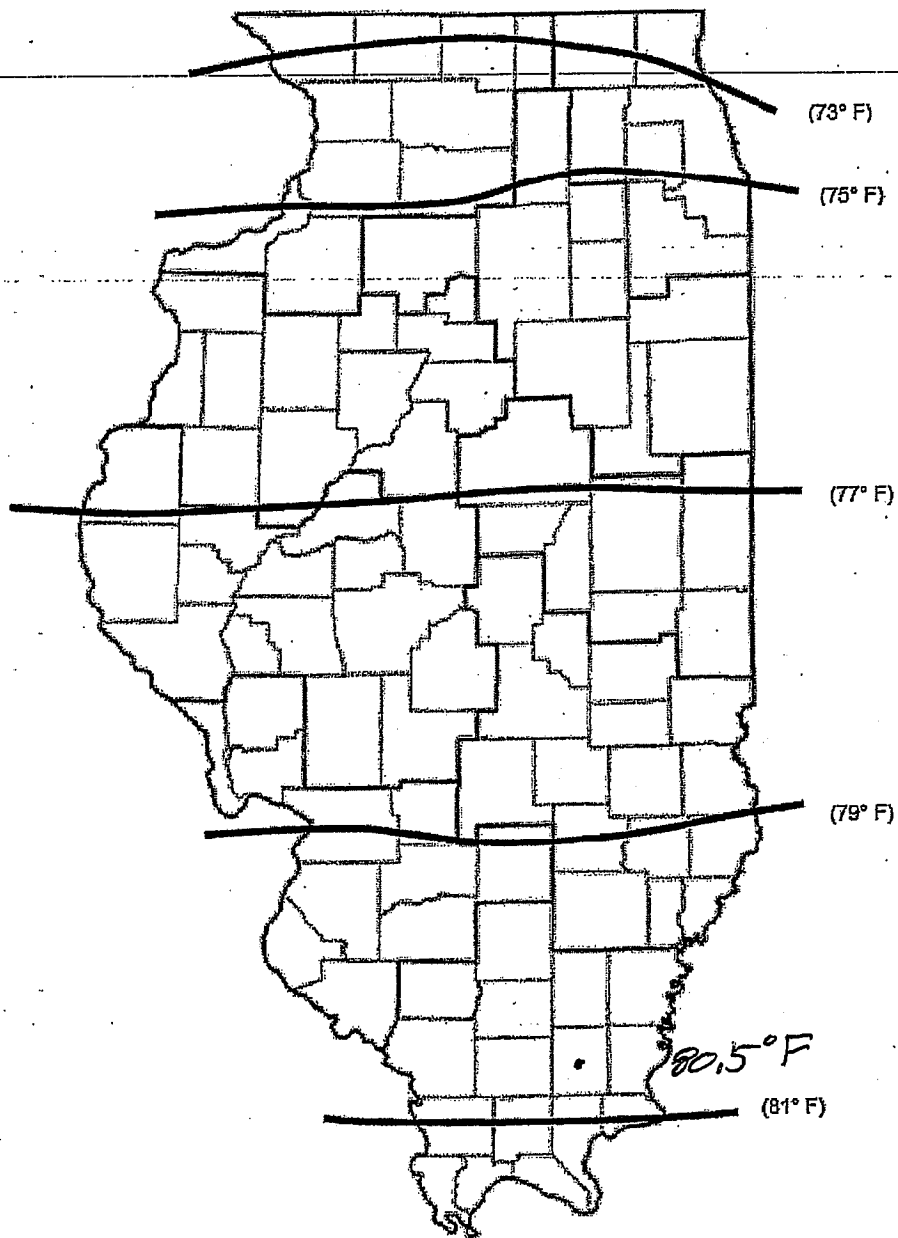
0.19 ✓    0.86 ✓                    1.00 ✓    1.00 ✓

EQUIVALENCY FACTOR (ESAL/VEH) = 3.05 ✓

MINE OPERATOR STATED THE MINE OPERATES 24 HOURS A DAY, 7 DAYS A WEEK - ASSUME HOLIDAYS ON NEW YEARS, 4<sup>TH</sup> OF JULY, MEMORIAL DAY, THANKS GIVING, & CHRISTMAS - 360 WORKING DAYS

$$\text{MINE TF} = \frac{900 \times 3.05 \times 360 \times 20}{1,000,000} = 19.76 \checkmark$$

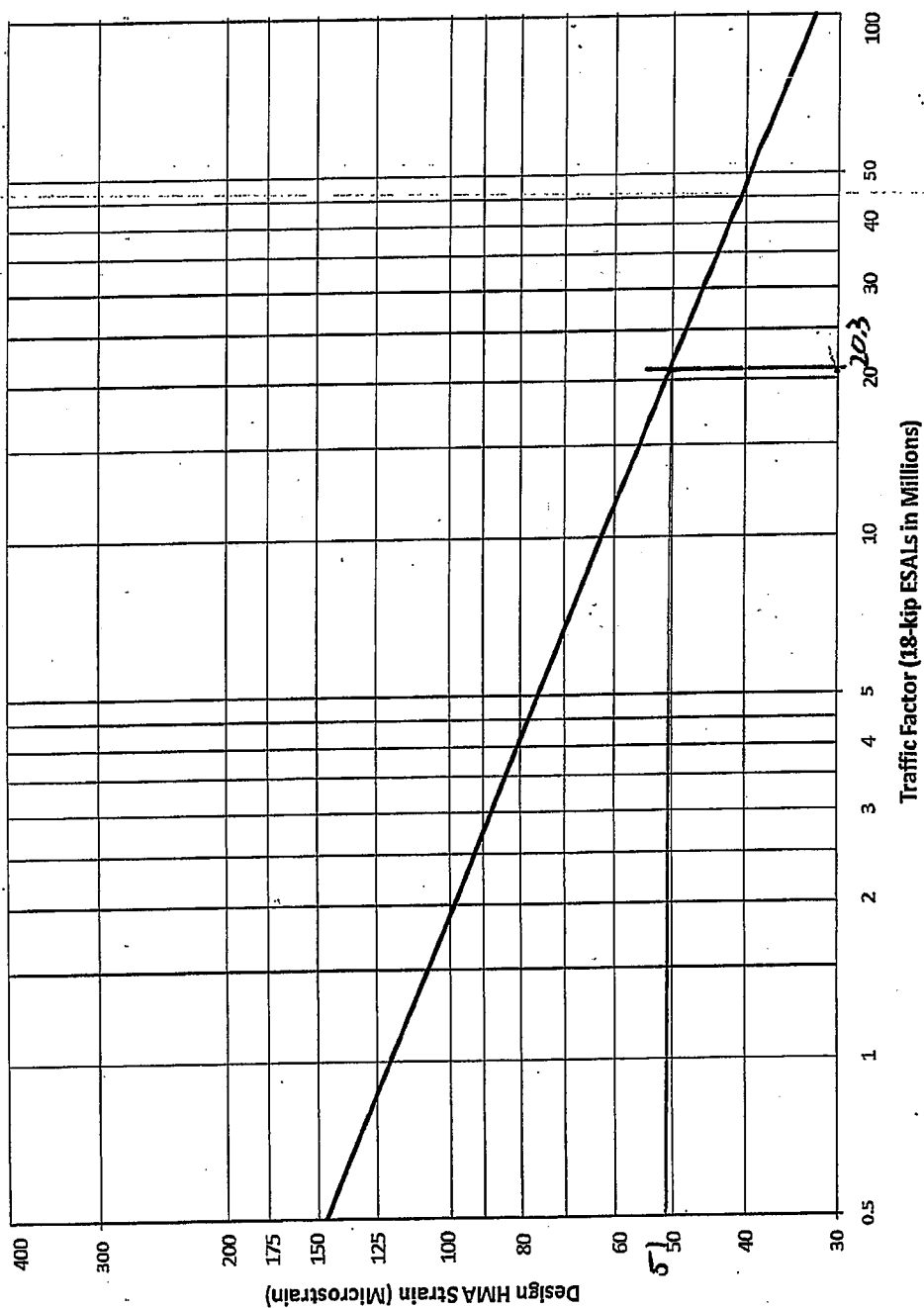
$$\text{DESIGN TF} = 19.76 + 0.53 = 20.29 \checkmark$$



*Note: The minimum design HMA mixture temperature will be 73°F.*

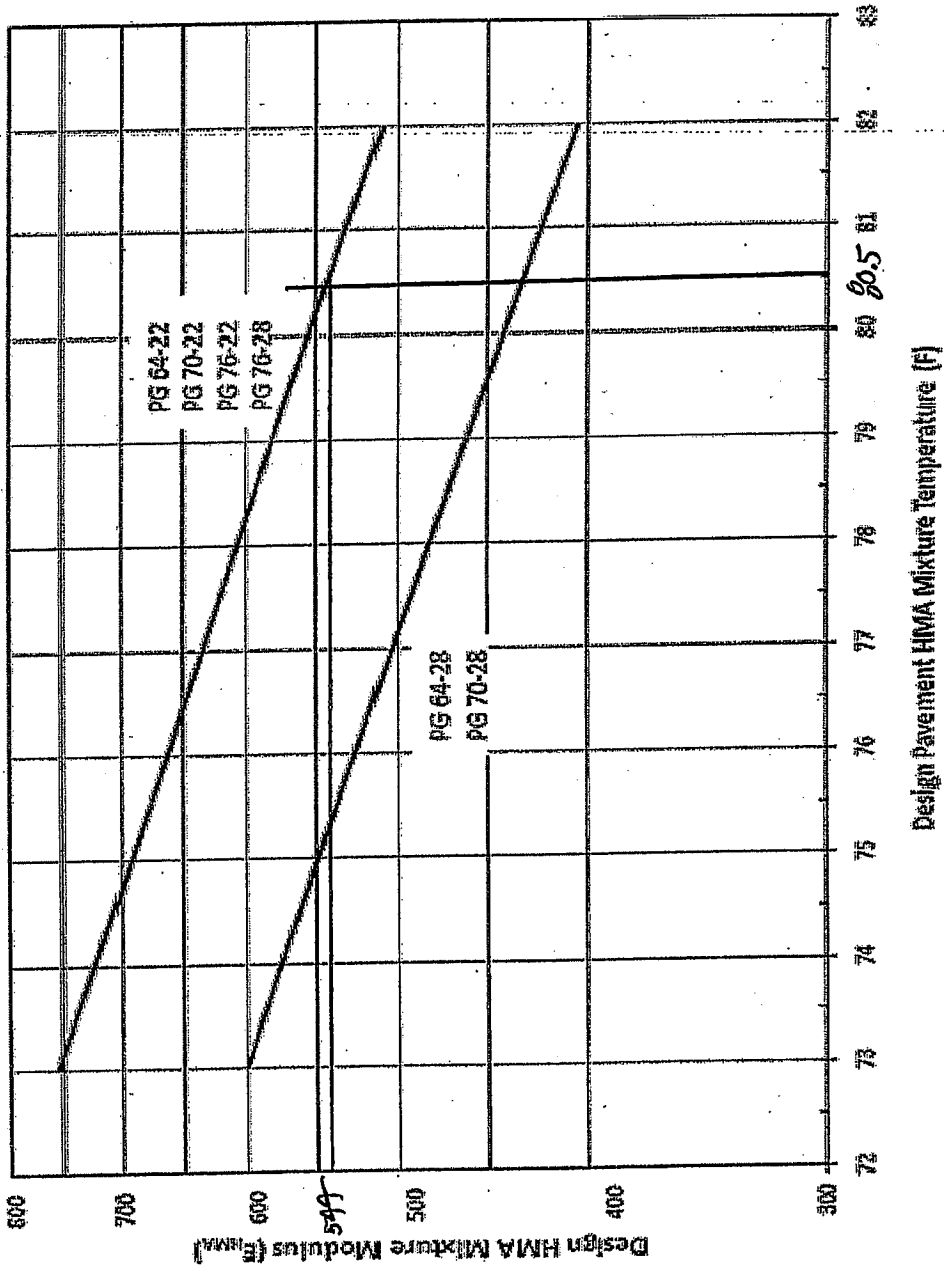
**HMA MIXTURE TEMPERATURE**  
**(Mechanistic Design: Flexible Pavement)**

Figure 54-5.C



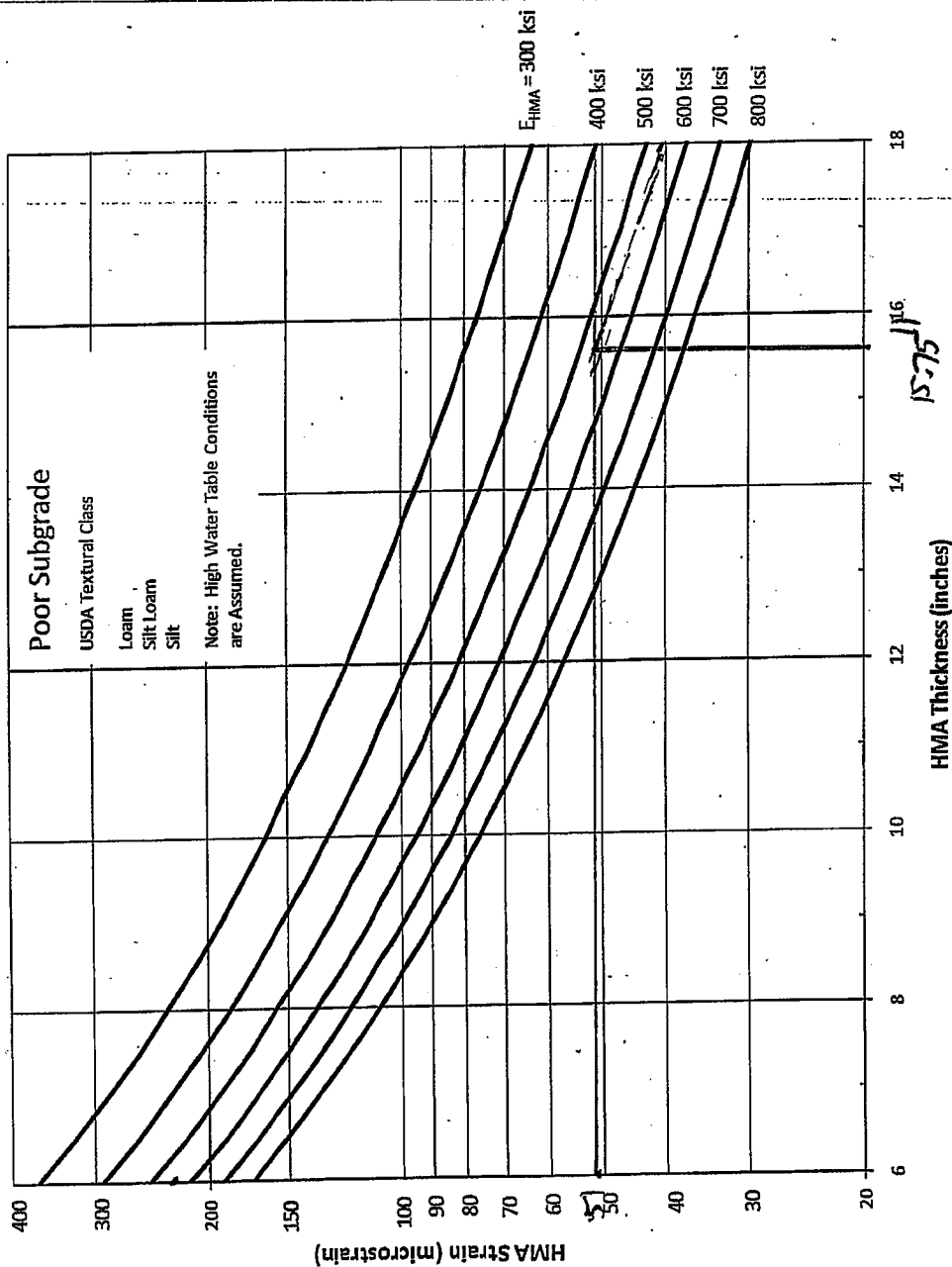
DESIGN HMA STRAIN  
(Mechanistic Design: Flexible Pavement)

Figure 54-5.E



HMA MIXTURE MODULUS (E<sub>HMA</sub>)  
(Mechanistic Design: Flexible Pavement)

Figure 54-5.D



**HMA THICKNESS DESIGN CHART**  
(Mechanistic Design: Flexible Pavement: SSR = Poor)

Figure 54-5.F



SSR = POOR

BINDER GRADE = PG 64-22

MIX TEMPERATURE = 80.5° ✓ FIG 54-5C

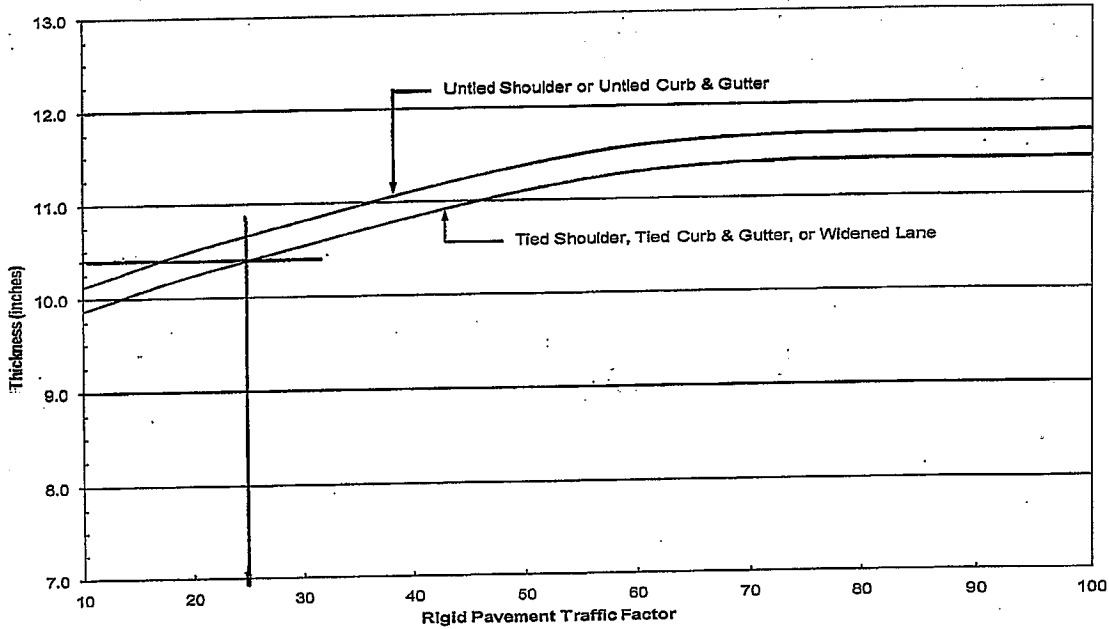
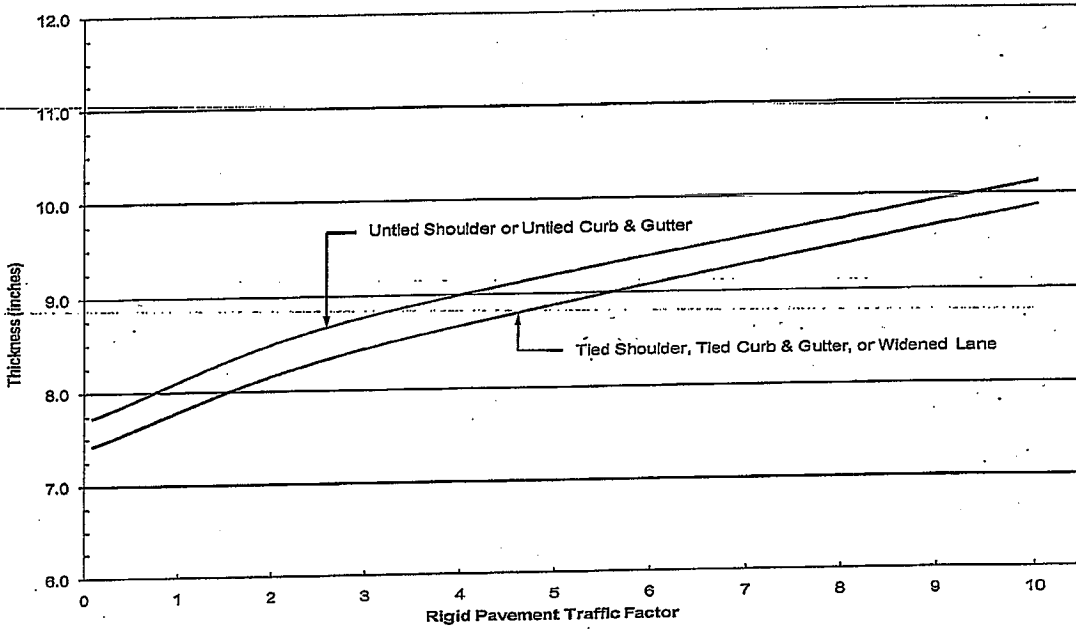
$E_{HMA} = 544$  ✓ FIG 54-5D

HMA STRAIN = 51 ✓ FIG 54-5E

THICKNESS = 15.75" ✓ FIG 54-5F

LIMITING STRAIN = 16.75" ✓ FIG 54-5I

USE 15.75" ✓



Note: Use of untied shoulder design requires BDE approval.

use 10.5"

**RIGID PAVEMENT DESIGN CHART**  
(Mechanistic Design: SSR = Poor)

Figure 54-4.E

# PCC PAVEMENT DESIGN

TIED SHOULDER

SHEET 4 OF 4

$$TF = 20 \left[ \frac{(0.15)(0.5)(3866) + (135.78)(0.5)(298) + (567.2)(0.5)(66)}{1,000,000} \right]$$

$$TF = 0.72 \checkmark$$

USE THE SAME AXLE LOAD DISTRIBUTION AS  
HMA

RIGID, PAVEMENT ESAL VALUES (TABLE 1)  
0.19  $\checkmark$     1.49  $\checkmark$     1.00  $\checkmark$     1.00

EQUIVALENCY FACTOR (ESALIVEH) = 3.67 3.68

$$\text{MINE TF} = \frac{900 \times 3.67 \times 360 \times 20}{1,000,000} = 23.78 \quad 23.85$$

$$\text{DESIGN TF} = 23.78 + 0.72 = 24.50 \quad 24.57$$

POOR SOIL

THICKNESS 10.5"  $\checkmark$  FIG 54-4.E

## PAVEMENT DESIGN

CALC CWS

ILL 34 CLASS II STREET DATE 2-22-12

SHEET 1 OF 4

EXIST	2011 ADT	RAW DATA 6-29-11	RAW DATA % MIDE	2012 (1.50%)
ADT	4300	4604	4504	5225
SU	215	214	214	248
MU	900	1057	57	66
PV	3185	3333	3333	3866

MINE OPERATOR ESTIMATES THEY WILL BE  
SENDING OUT 900 TRUCKS PER DAY BY  
JUNE 2012 - ASSUME THIS IS THE MAX  
CAPACITY FOR THE MINE - REDUCE RAW DATA  
MUS BY PRESENT MINE TRAFFIC OF 500 TRUCKS

$$\text{NORMAL TF} = 20 \left[ \frac{(0.15)(0.5)(3866) + (112.06)(0.5)(248) + (385.44)(0.5)(66)}{1,000,000} \right]$$

$$\text{NORMAL TF} = 0.53$$

SHEET 2 OF 4

MINE OPERATOR STATED TRUCKS WERE LOADED SO THAT THE TOTAL WEIGHT OF THE TRUCK IS 80,000 LBS.

ASSUME TYPICAL AXLE LOAD DISTRIBUTION IS THE SAME AS THE EXAMPLE

12K    32K                    18K    18K  
○    ○○                    ○    ○

FLEXIBLE PAVEMENT ESAL VALUES (TABLE 1)

0.19    0.86                    1.00    1.00

EQUIVALENCY FACTOR (ESAL/VEH) = 3.05

MINE OPERATOR STATED THE MINE OPERATES 24 HOURS A DAY, 7 DAYS A WEEK - ASSUME HOLIDAYS ON NEW YEARS, 4<sup>TH</sup> OF JULY, MEMORIAL DAY, THANKS GIVING, & CHRISTMAS - 360 WORKING DAYS

$$\text{MINE TF} = \frac{900 \times 3.05 \times 360 \times 20}{1,000,000} = 19.76$$

$$\text{DESIGN TF} = 19.76 + 0.53 = 20.29$$

SHEET 3 OF 4

SSR = POOR

BINDER GRADE = PG 64-22

MIX TEMPERATURE =  $80.5^{\circ}$  FIG 54-5.C

$E_{HMA}$  = 544 FIG 54-5.D

HMA STRAIN = 51 FIG 54-5.E

THICKNESS = 15.75" FIG 54-5.F

LIMITING STRAIN = 16.75" FIG 54-5.I

USE 15.75"

# PCC PAVEMENT DESIGN

TIED SHOULDER

SHEET 4 OF 4

$$TF = 20 \left[ \frac{(0.15 \times 0.5)(3866) + (135.78)(0.5)(248) + (567.21)(0.5)(66)}{1,000,000} \right]$$

$$TF = 0.72$$

USE THE SAME AXLE LOAD DISTRIBUTION AS  
HMA

RIGID PAVEMENT ESAL VALUES (TABLE 1)

0.19      1.49      1.00      1.00

EQUIVALENCY FACTOR (ESAL/VEH) = 3.67

$$\text{MINE TF} = \frac{900 \times 3.67 \times 360 \times 20}{1,000,000} = 23.78$$

$$\text{DESIGN TF} = 23.78 + 0.72 = 24.50$$

POOR SOIL

THICKNESS 10.5"      FIG. 54-4.E

**PROJECT AND TRAFFIC INPUTS**

(Enter Data in Gray Shaded Cells)

Route: ILL 34	Comments:		
Section:	Design Date: 02/14/2012	C Stein <-- BY	
County: Saline	Modified Date:		
Location: North of Harrisburg			
Facility Type Other Marked State Route			
# of Lanes = 2 or 3			
Part of future 4 lanes or more ? No			
One Way Street ? No			
Road Class: II			
Subgrade Support Rating (SSR): Poor			
Construction Year: 2012			
Design Period (DP) = 20 years			

	ADT	Year
Current:	-	-
Future:	-	-

	Structural Design Traffic			
	Minimum ADT	Actual ADT	Actual % of Total ADT	% of ADT in Design Lane
PV =	0	3,866	92.5%	P = 50%
SU =	250	248	5.9%	S = 50%
MU =	750	66	1.6%	M = 50%
	Struct. Design ADT = 4,180			(2022)

**TRAFFIC FACTOR CALCULATION**

<b>FLEXIBLE PAVEMENT</b>		<b>RIGID PAVEMENT</b>	
Cpv =	-	Cpv =	-
Csu =	-	Csu =	-
Cmu =	-	Cmu =	-
TF flexible (Actual) =	-	(Actual ADT)	TF rigid (Actual) =
TF flexible (Min) =	-	(Min ADT, Fig 54-2.C)	TF rigid (Min) =

**ADDITIONAL DESIGN INPUTS AND FLEXIBLE & RIGID PAVEMENT THICKNESS CALCULATIONS**

<b>HMA Pavement</b>	<b>JPCP Pavement</b>
Use TF flexible = 20.29	Use TF rigid = 24.50
PG Grade Lower Binder Lifts = PG 64-22 (Figure 53-4.R)	Edge Support = Tied Shoulder or C.&G.
HMA Mixture Temp. = 80.5 deg. F (Figure 54-5.C)	<b>Rigid Pavt Thick. = 10.40 in. (Figure 54-4.E)</b>
Design HMA Mixture Modulus (E <sub>HMA</sub> ) = 544 ksi (Figure 54-5.D)	10.50
Design HMA Strain (ε <sub>HMA</sub> ) = 51 (Figure 54-5.E)	
Full Depth HMA Design Thickness = 15.53 in. (Figure 54-5.F)	
Limiting Strain Criterion Thickness = 16.75 in. (Figure 54-5.I)	
<b>Use Full Depth HMA Thickness = 15.53 inches</b>	15.75

**DESIGN TABLES FROM BDE MANUAL CHAPTER 54 - PAVEMENT DESIGN**

<b>Class I Roads</b> 4 lanes or more Part of a future 4 lanes or more One-way Streets with ADT > 3500	<b>Class II Roads</b> 2 lanes with ADT > 2000 One way Street with ADT <= 3500	<b>Class III Roads</b> 2 Lanes (ADT 750 -2000)	<b>Class IV Roads</b> 2 Lanes (ADT < 750)
--	---	--	---

	Min. Str. Design Traffic (Fig 54-2.C)		
Facility Type	PV	SU	MU
Interstate or Supplemental Freeway	0	500	1500
Other Marked State Route	0	250	750
Unmarked State Route	No Min	No Min	No Min

	Traffic Factor ESAL Coefficients			
	Rigid (Fig. 54-4.C)		Flexible (Fig. 54-5.B)	
Class	Csu	Cmu	Csu	Cmu
I	143.81	696.42	132.50	482.53
II	135.78	567.21	112.06	385.44
III	129.58	562.47	109.14	384.35
IV	129.58	562.47	109.14	384.35

	Class Table for One-Way Streets	
	ADT	Class
	0 - 3500	II
	>3501	I

	Class Table for 2 or 3 lanes (not future 4 lane & not one-way street)	
	ADT	Class
	0 - 749	IV
	750 - 2000	III
	>2000	II

	Design Lane Distribution Factors For Structural Design Traffic (Figure 54-2.B)					
	Rural			Urban		
Number of Lanes	P	S	M	P	S	M
1 Lane Ramp	100%	100%	100%	100%	100%	100%
2 or 3	50%	50%	50%	50%	50%	50%
4	32%	45%	45%	32%	45%	45%
6 or more	20%	40%	40%	8%	37%	37%



