



Illinois Department of Transportation

Memorandum

To:	Jack Elston	Attn:	Michael Brand
From:	Kensil A. Garnett	By:	Ryan T. Carroll
Subject:	Pavement Analysis*		
Date:	August 22, 2019		

*Route: FAI 55 (I-55)
Section: D5 NHFP 2020-1
County: McLean
Contract: 70D73
Location: Funks Grove Rest Area
Improvement Type: NB Truck Parking Expansion
Letting: April 2020

The District has completed the pavement analysis for the above-mentioned project. Review by BDE is required since the total pavement area for construction exceeds 4,750 Square Yards.

The following is the scope of the project:

Northbound truck parking expansion at the Funks Grove Rest Area.

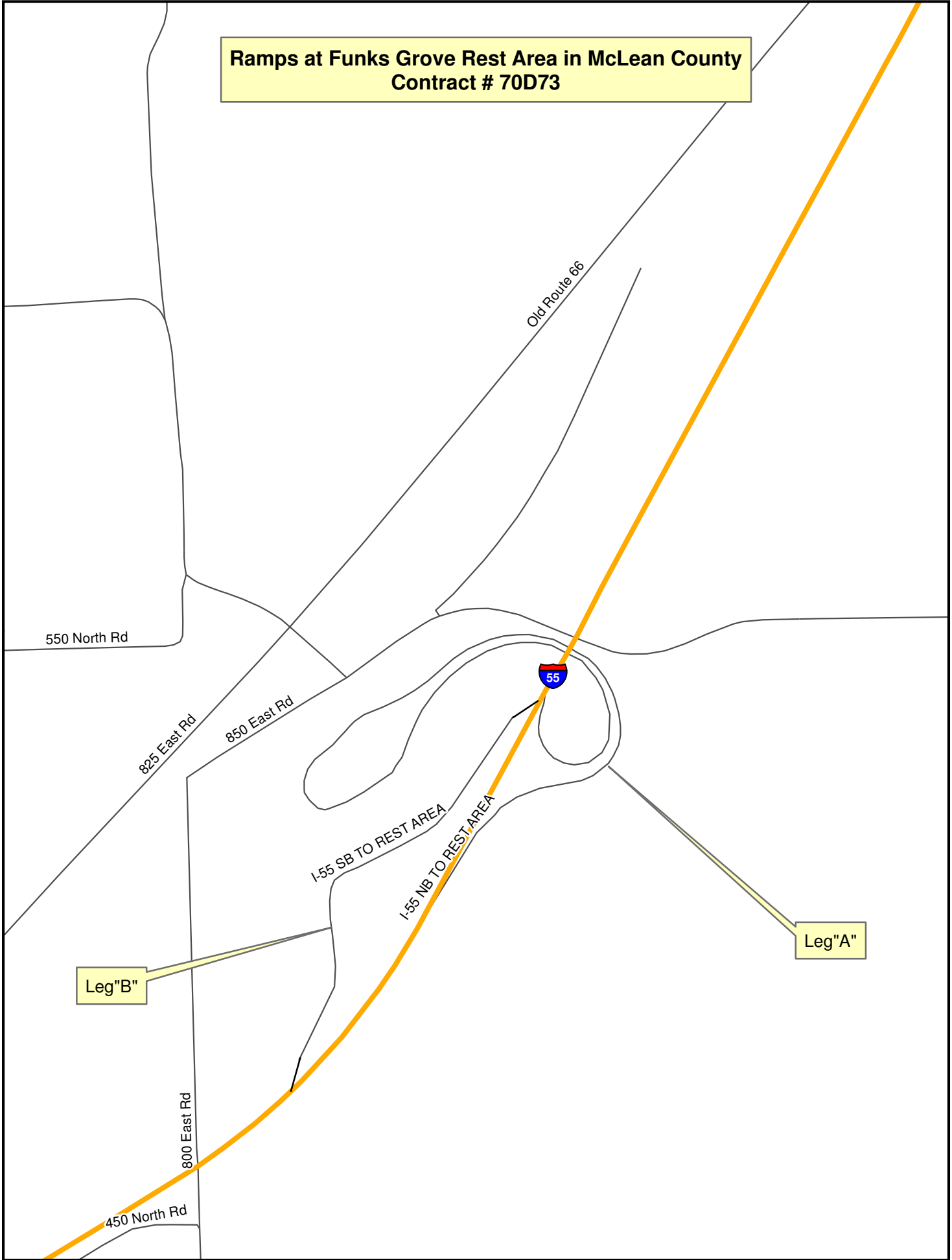
A 20-year pavement analysis was performed for the above parking expansion. The location is considered a "High Stress" location and will be classified as a "Special Design" per BDE Figure 54-1.A. A mechanistic-rigid pavement design is recommended to match the existing adjacent truck parking pavement type.

The pavement design consists of the following:

- 10.50" JPC
- 4" Stabilized Subbase
- 12" Aggregate Subgrade Improvement

If you have any questions or need additional information, please contact Ryan Carroll at (217)466-7225.

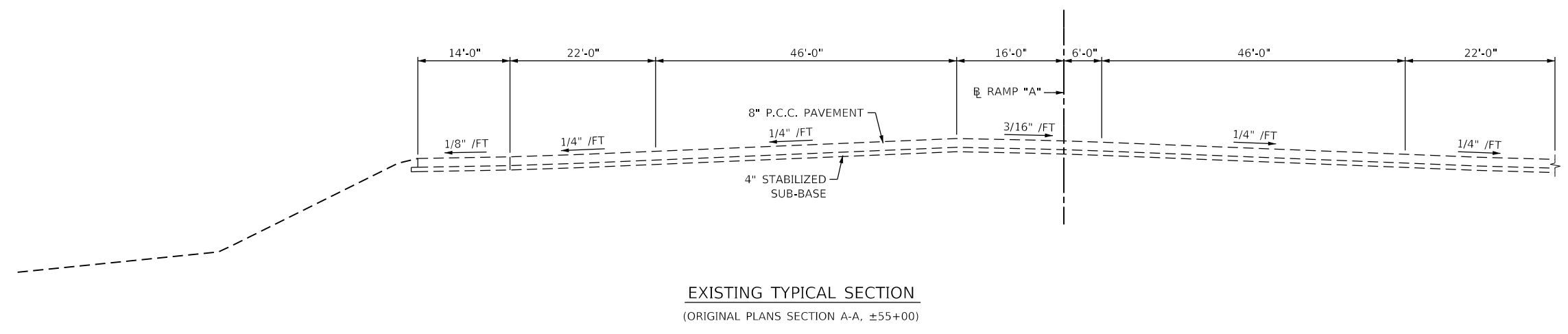
**Ramps at Funks Grove Rest Area in McLean County
Contract # 70D73**



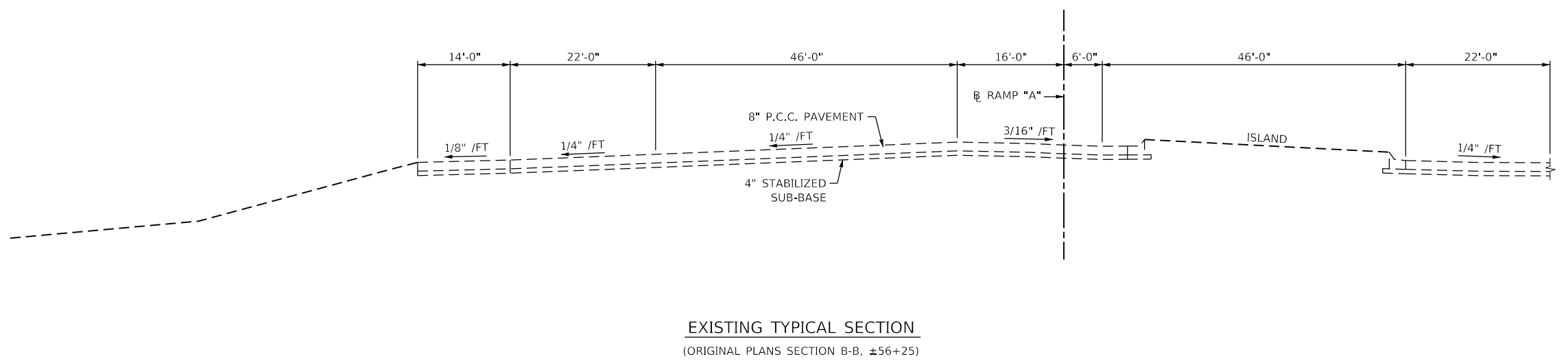


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EXISTING TYPICAL SECTION
 (ORIGINAL PLANS SECTION A-A, ±55+00)



EXISTING TYPICAL SECTION
 (ORIGINAL PLANS SECTION B-B, ±56+25)

Prairie Engineers, P.C.
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 Professional Design Firm No. 154-005965
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

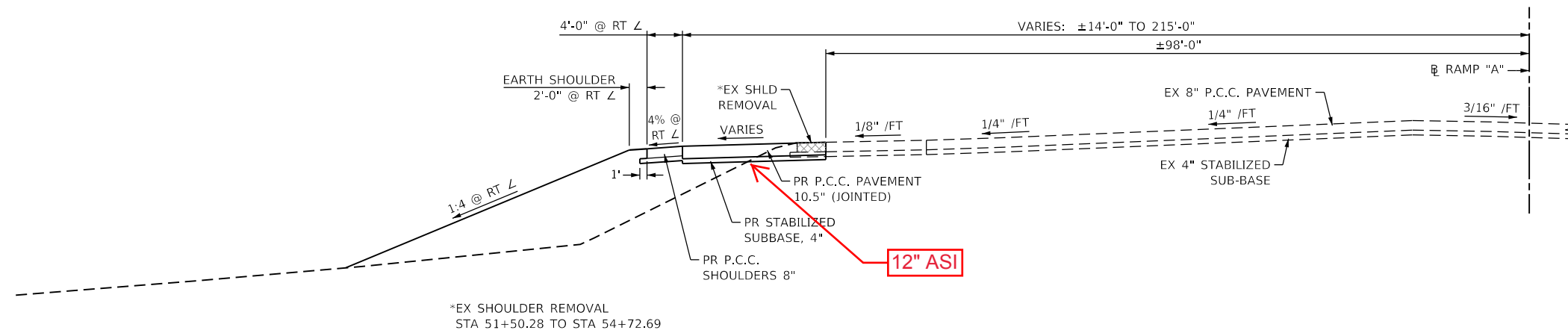
NB PARKING EXPANSION AT FUNKS GROVE REST AREA
TYPICAL CROSS SECTIONS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
55	D5 NHFP 2020-1	MCLEAN		
CONTRACT NO. 70D73				
		ILLINOIS	FED. AID PROJECT	

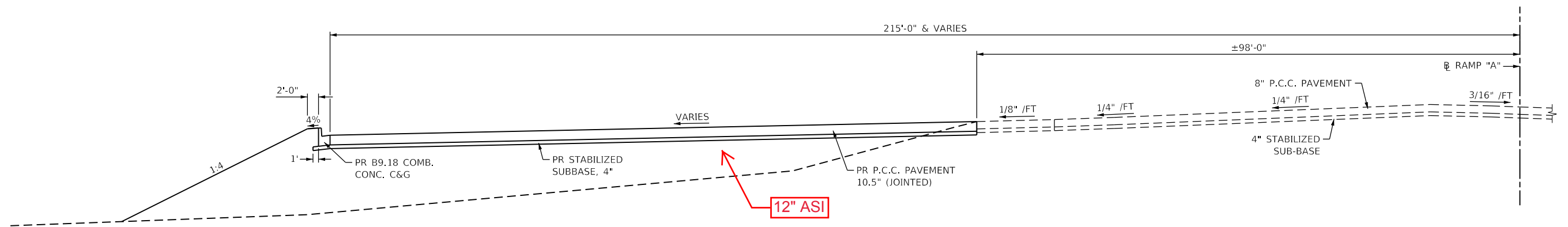
SCALE: SHEET 1 OF SHEETS STA. TO STA.

MODEL: 02 - Proposed Typical

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PROPOSED TYPICAL SECTION
 STA 51+50.28 TO STA 55+34.88



PROPOSED TYPICAL SECTION
 STA 55+34.88 TO STA 58+70.44
 STA 60+00.10 (AH) TO STA 61+31.66 (AH)

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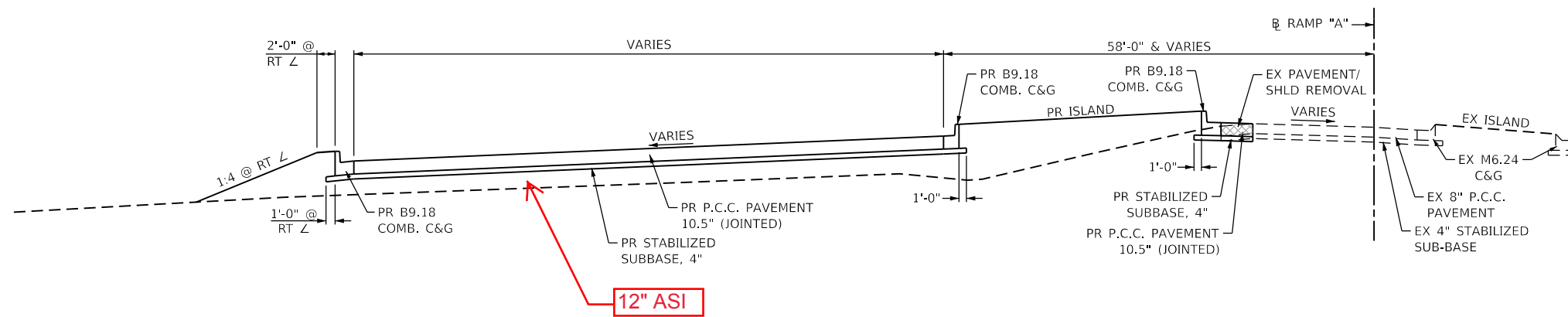
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	DATE -	REVISED -

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NB PARKING EXPANSION AT FUNKS GROVE REST AREA
TYPICAL CROSS SECTIONS

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
55	D5 NHFP 2020-1	MCLEAN		
CONTRACT NO. 70D73				
ILLINOIS FED. AID PROJECT				

SCALE: SHEET 2 OF SHEETS STA. TO STA.



PROPOSED TYPICAL SECTION

STA 58+70.44 TO STA 60+00.10 (AH)

**STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION**

**NB PARKING EXPANSION AT FUNKS GROVE REST AREA
TYPICAL CROSS SECTIONS**

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
55	D5 NHFP 2020-1	MCLEAN		
CONTRACT NO. 70D73				
		ILLINOIS	FED. AID PROJECT	

SCALE: SHEET 3 OF SHEETS STA. TO STA.

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PLOT DATE = 5/3/2019	DATE -	REVISED -

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5/3/2019 8:19:37 AM bbrennan I:\Jobs\2018\911804_PTB_189-024_District_5_Various Phase 1\1\W\2_Funks_Grove\CAD_Sheets\0570D73-ent-typical.dgn MODEL: 03 Proposed Typicals

PROJECT AND TRAFFIC INPUTS

(Enter Data in Gray Shaded Cells)

Route: **FAI 55**
 Section: **D5 NHFP 2020-1**
 County: **MCLEAN**
 Location: **FUNKS GROVE NB TRUCK PARKING**

Comments:
 Design Date: **04/26/2019** **RTC**
 Modify Date:

<-- BY	ADT	Year
Current:	25,100	2018
Future:	28,100	2038

Facility Type: **Interstate or Freeway**
 # of Lanes = **4**

Road Class: **I**
 Subgrade Support Rating (SSR): **Poor**
 Construction Year: **2021**
 Design Period (DP) = **20** years

	Structural Design Traffic			% of ADT in Design Lane
	Minimum ADT	Actual ADT	Actual % of Total ADT	
PV =	0	18,340	67.8%	P = 16%
SU =	500	974	3.6%	S = 25%
MU =	1500	7,736	28.6%	M = 25%
Struct. Design ADT =	27,050	(2031)		

TRAFFIC FACTOR CALCULATION

FLEXIBLE PAVEMENT		RIGID PAVEMENT	
Cpv =	0.15	Cpv =	0.15
Csu =	132.5	Csu =	143.81
Cmu =	482.53	Cmu =	696.42
TF flexible (Actual) =	19.32 (Actual ADT)	TF rigid (Actual) =	27.65 (Actual ADT)
TF flexible (Min) =	3.95 (Min ADT Fig. 54-2.C)	TF rigid (Min) =	5.58 (Min ADT Fig. 54-2.C)

NEW CONSTRUCTION / RECONSTRUCTION PAVEMENT DESIGN CALCULATIONS

Full-Depth HMA Pavement	JPC Pavement
Use TF flexible = 19.32	Use TF rigid = 27.65
PG Grade Lower Binder Lifts = PG 64-22 (Fig. 53-4.R)	Edge Support = Tied Shoulder or C.&G.
HMA Mixture Temp. = 76.0 deg. F (Fig. 54-5.C)	Rigid Pavt Thick. = 10.50 in. (Fig. 54-4.E)
Design HMA Mixture Modulus (E _{HMA}) = 660 ksi (Fig. 54-5.D)	
Design HMA Strain (ε _{HMA}) = 52 (Fig. 54-5.E)	CRC Pavement
Full Depth HMA Design Thickness = 14.00 in. (Fig. 54-5.F)	Use TF rigid = 27.65
Limiting Strain Criterion Thickness = 15.50 in. (Fig. 54-5.I)	IBR value = 3
Use Full-Depth HMA Thickness = 14.00 inches	CRCP Thickness = 10.25 in. (Fig. 54-4.M)

TF MUST BE > 60 FOR CRCP

RECONSTRUCTION ONLY (SUPPLEMENTAL) PAVEMENT DESIGN CALCULATIONS

HMA Overlay of Rubblized PCC	Unbonded Concrete Overlay
Use TF flexible = 19.32	Review 54-4.03 for limitations and special considerations.
HMA Overlay Design Thickness = 11.25 in. (Fig. 54-5.U)	
Limiting Strain Criterion Thickness = 11.00 in. (Fig. 54-5.V)	
Use HMA Overlay Thickness = 11.00 inches	JPCP Thickness = NA inches

CONTACT BMPR FOR ASSISTANCE

DESIGN TABLES FROM BDE MANUAL CHAPTER 54 - PAVEMENT DESIGN

Class I Roads	Class II Roads	Class III Roads	Class IV Roads
4 lanes or more Part of a future 4 lanes or more One-way Streets with ADT > 3500	2 lanes with ADT > 2000 One way Street with ADT <= 3500	2 Lanes (ADT 750 -2000)	2 Lanes (ADT < 750)

Facility Type	Min. Str. Design Traffic (Fig 54-2.C)		
	PV	SU	MU
Interstate or Freeway	0	500	1500
Other Marked State Route	0	250	750
Unmarked State Route	No Min	No Min	No Min

Class Table for One-Way Streets	
ADT	Class
0 - 3500	II
>3501	I

Class	Traffic Factor ESAL Coefficients			
	Rigid (Fig. 54-4.C)		Flexible (Fig. 54-5.B)	
	Csu	Cmu	Csu	Cmu
I	143.81	696.42	132.50	482.53
II	135.78	567.21	112.06	385.44
III	129.58	562.47	109.14	384.35
IV	129.58	562.47	109.14	384.35

Class Table for 2 or 3 lanes (not future 4 lane & not one-way street)	
ADT	Class
0 - 749	IV
750 - 2000	III
>2000	II

Number of Lanes	Design Lane Distribution Factors For Structural Design Traffic (Fig. 54-2.B)					
	Rural			Urban		
	P	S	M	P	S	M
1 Lane Ramp	100%	100%	100%	100%	100%	100%
2 or 3	50%	50%	50%	50%	50%	50%
4	32%	45%	45%	32%	45%	45%
6 or more	20%	40%	40%	8%	37%	37%