

11-06-2015 LETTING ITEM 052

**MARION**  
STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS

**PROPOSED  
HIGHWAY PLANS**

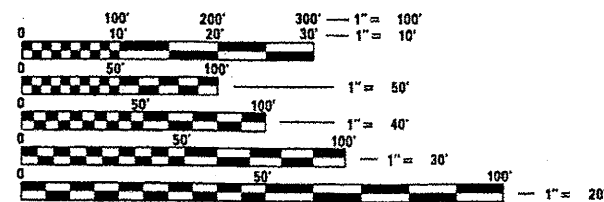
FAP ROUTE 327 A (US 50)  
SECTION 15BR  
PROJECT ACNHF-0327 (064)  
STRUCTURE REPLACEMENT  
MARION COUNTY

C-98-041-12

FOR INDEX OF SHEETS, SEE SHEET NO. 2

100%  
10-29-2016

DESIGN DESIGNATION  
N/A



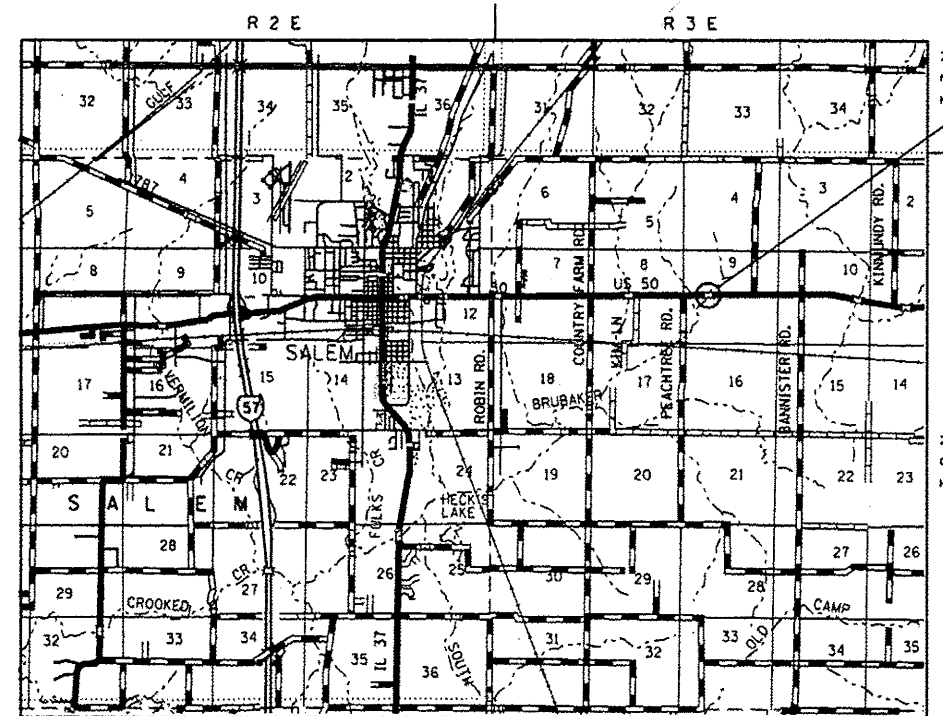
FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD  
ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT  
CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS  
ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.  
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION  
1-800-892-0123  
OR 811

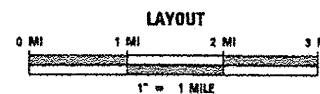
PROJECT ENGINEER: BILLIE OWEN (618) 346-3209  
SQUAD CONTACT: JON DINTELMAN (618) 346-3195

CONTRACT NO. 76A23

061-0093



PROPOSED SINGLE SPAN SLAB  
BRIDGE OVER BRUBAKER CREEK  
43'-0" BACK TO BACK OF ABUTMENT  
0 DEGREE SKEW STATION 39+39.00  
RESURFACING LIMITS:  
BEGIN STA. 36+90.00  
END STA. 42+00.00

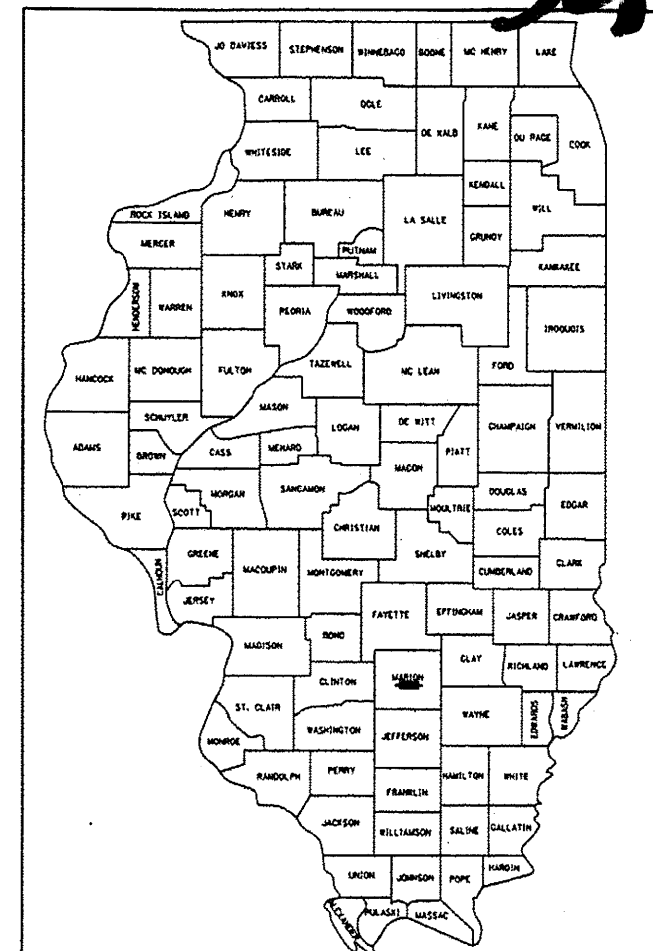


GROSS LENGTH = 43 FT. = 0.008 MILE  
NET LENGTH = 43 FT. = 0.008 MILE

LATITUDE: 38.62808  
LONGITUDE: -88.87883

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
327	15BR	MARION	57	1
ILLINOIS CONTRACT NO. 76A23				

D-98-017-06



LOCATION OF SECTION INDICATED THUS: -

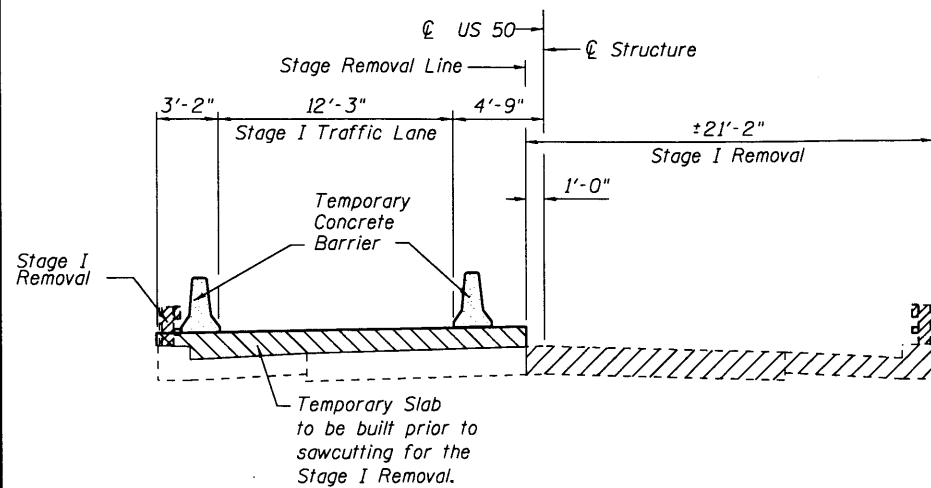
2012 ADT 5200  
2022 ADT 5700  
SU = 4.9%  
MU = 11.2%

STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS

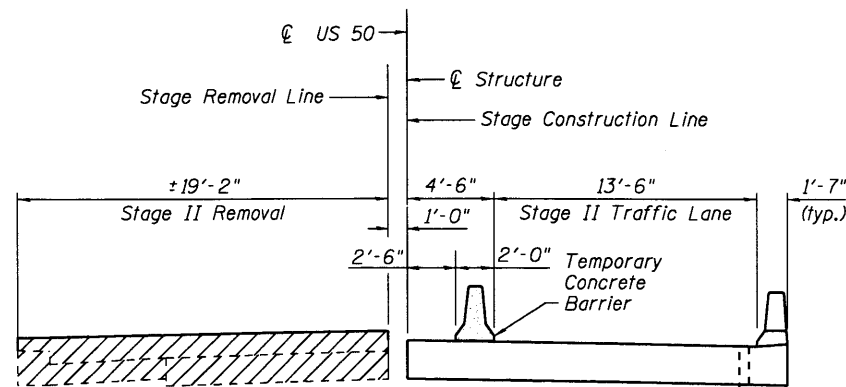
SUBMITTED Aug 14 2015  
John D. Baranzoni, P.E.  
DEPUTY DIRECTOR OF HIGHWAYS, REGION 5 ENGINEER  
Oct 2 2015  
John D. Baranzoni, P.E.  
ENGINEER OF DESIGN AND ENVIRONMENT  
Oct 2 2015  
Omer Osman, P.E.  
DIRECTOR OF HIGHWAYS, CHIEF ENGINEER

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OF THE STATE OF ILLINOIS

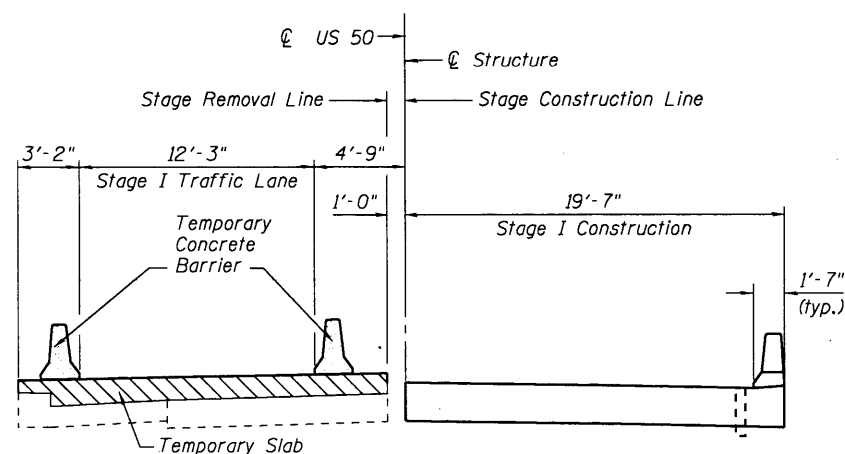




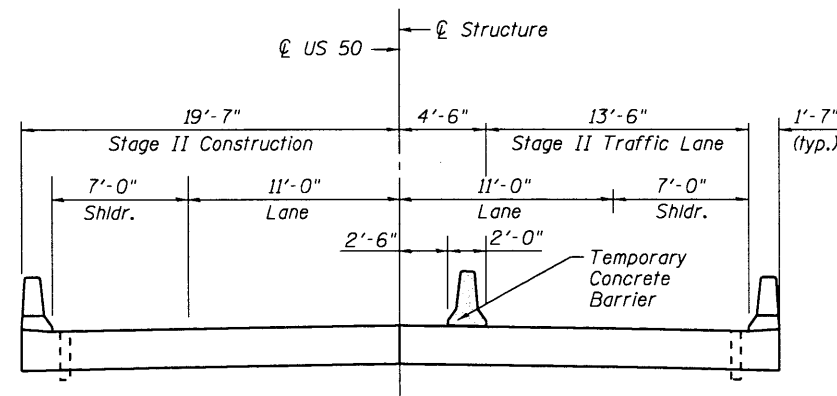
### STAGE I REMOVAL



### STAGE II REMOVAL



### STAGE I CONSTRUCTION



## STAGE II CONSTRUCTION

NOTES

1. All staging cross sections are looking West.
2. For quantity of Temporary Concrete Barrier, see roadway plans.
3. If the Contractor chooses to alter the temporary cantilevered sheet piling design requirements shown on the plans, a design submittal including plan details and calculations signed and sealed by an Illinois Structural Engineer will be required for review and acceptance by the Engineer.
4. The Contractor shall connect the first sheet to the existing abutment wall to ensure stability of sheets driven to the top of the existing footing. This connection shall be reviewed and accepted by the Engineer and included in the cost for Temporary Sheet Piling.

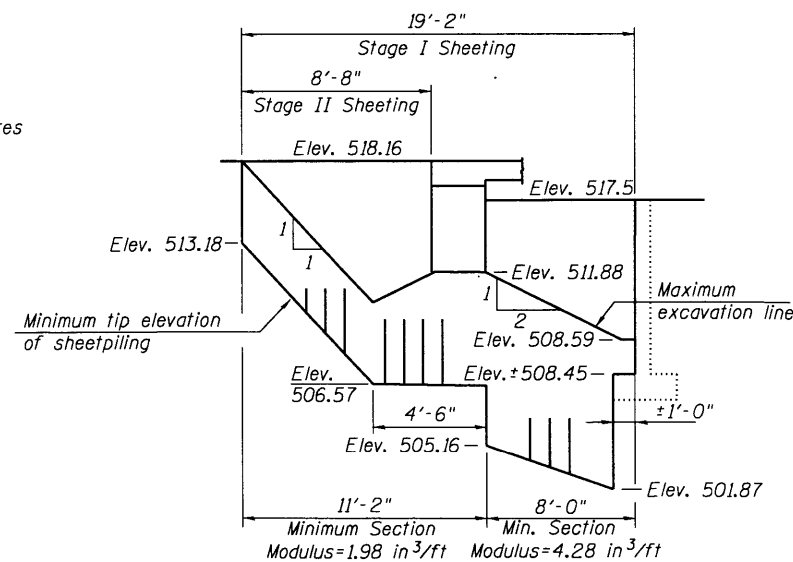
LEGEND



### Removal of Existing Structures



*Temporary Slab*



TEMPORARY SHEETING DETAIL

TOTAL BILL OF MATERIAL

Item	Unit	Super	Sub.	Total
Granular Backfill for Structures	Cu. Yd.		83	83
Stone Riprap, Class A5	Sq. Yd.		376	376
Filter Fabric	Sq. Yd.		376	376
Removal of Existing Structures	Each			1
Structure Excavation	Cu. Yd.		188	188
Concrete Structures	Cu. Yd.		52.2	52.2
Concrete Superstructure	Cu. Yd.	270.5		270.5
Bridge Deck Grooving	Sq. Yd.	385		385
Protective Coat	Sq. Yd.	470		470
Reinforcement Bars, Epoxy Coated	Pound	50,230	6,960	57,190
Bar Splicers	Each	282	58	340
Furnishing Metal Shell Piles 14"x0.25"	Foot		259	259
Driving Piles	Foot		259	259
Test Pile Metal Shells	Each		1	1
Name Plates	Each	1		1
Pipe Underdrains for Structures 4"	Foot		104	104
Temporary Sheet Piling	Sq. Ft.		426	426
Floor Drains	Each	4		4
Geocomposite Wall Drain	Sq. Yd.		52	52

### GENERAL NOTES

1. Reinforcement bars designated (E) shall be epoxy coated.
2. Layout of slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
3. Excavation behind existing abutment walls shall be performed to balance front and back soil pressure before removing the existing superstructure. The Contractor shall sawcut the upper portion of the existing abutment at the stage removal line before Stage I removal to ensure the remaining portion will not be prematurely damaged.
4. The Contractor shall make allowance for the deflections of forms, shrinkage and settlement of falsework, in addition to allowance for dead load deflection. Forms for deck slab shall be removed prior to placement of bridge approach slab.
5. The Contractor is advised that the existing bridge slab is in a deteriorated condition with a reduced load-carrying capacity. It is the Contractor's responsibility to account for the condition of the slab when developing construction procedures for stage construction and removal of the existing superstructure.

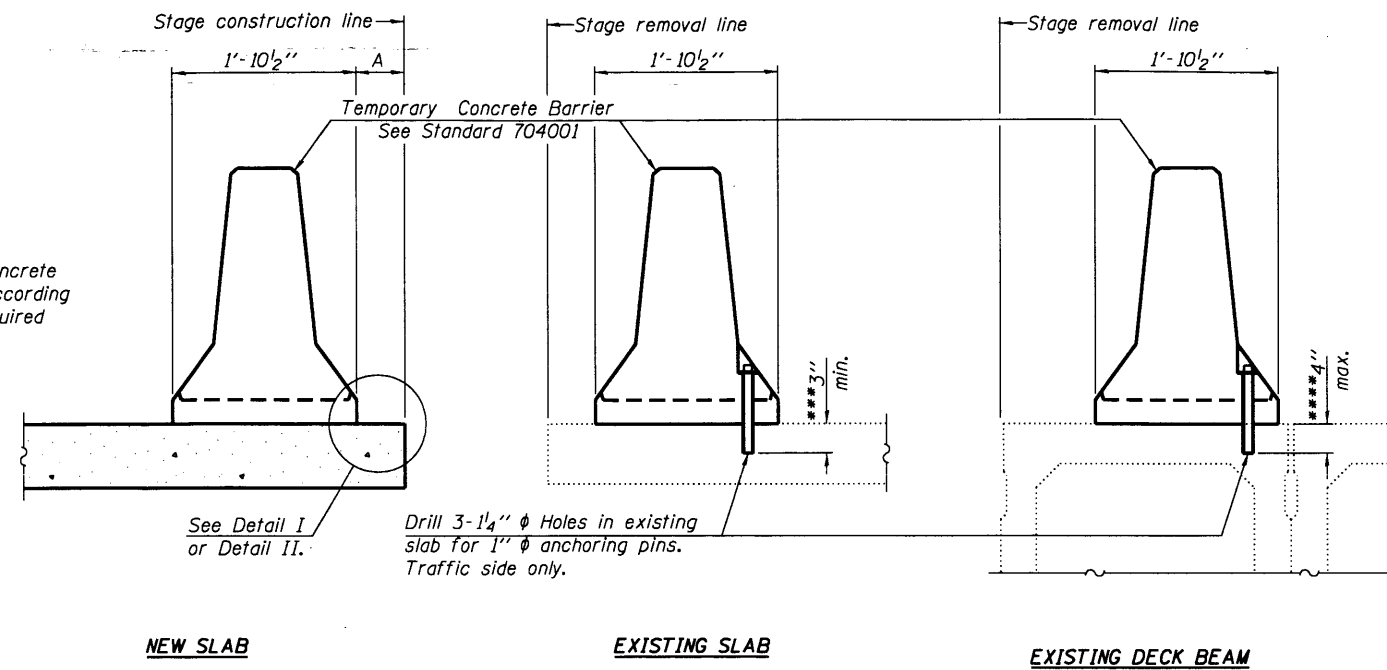
**Tran**Systems

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Suite 410  
St. Louis, MO 63102  
(314) 296-6765  
(314) 241-5164 Fax

FILE NAME = 0610093-76A23-002-General Data.dgn	USER NAME = mjrker	DESIGNED - SDG	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	GENERAL DATA STRUCTURE NO. 061-0093	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
		CHECKED - JMT	REVISED -			327	15-BR	MARION	57	31	
	PLOT SCALE =	DRAWN - SDG	REVISED -			CONTRACT NO. 76A23					
	PLOT DATE = 10/1/2015	CHECKED - JMT	REVISED -			SHEET NO. 2 OF 16 SHEETS					
	ILLINOIS FED. AID PROJECT										



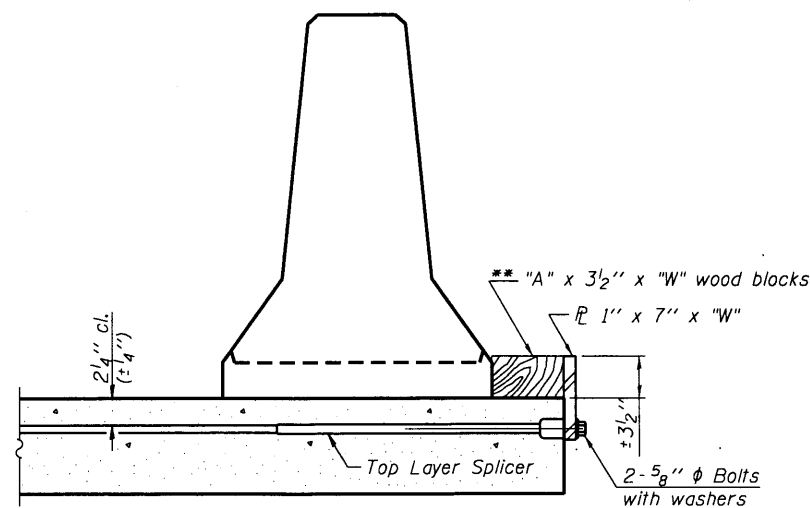
When "A" is 3'-1" or less, the temporary concrete barrier shall be anchored to the new slab according to Detail I or Detail II. No anchorage is required when "A" is greater than 3'-1".



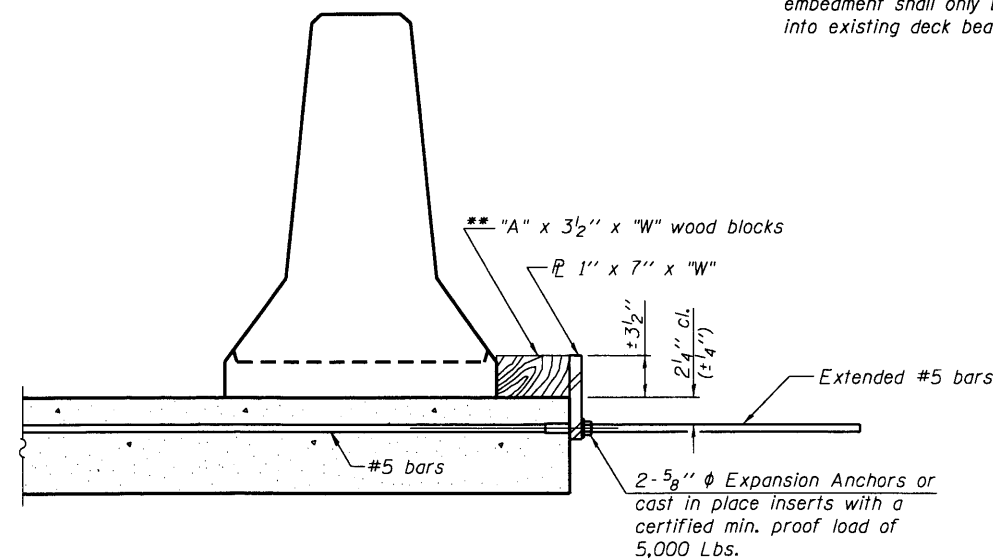
### SECTIONS THRU SLAB OR DECK BEAM

- NOTES**
- Detail I - With Bar Splicer or Couplers:  
Connect one (1) 1" x 7" x "W" steel  $\bar{P}$  to the top layer of couplers with 2-5/8"  $\phi$  bolts screwed to coupler at approximate  $\bar{C}$  of each barrier panel.
- Detail II - With Extended Reinforcement Bars:  
Connect one (1) 1" x 7" x "W" steel  $\bar{P}$  to the concrete slab or concrete wearing surface with 2-5/8"  $\phi$  Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate  $\bar{C}$  of each barrier panel.
- Cost of retainer assembly is included with Temporary Concrete Barrier. The 1" x 7" x "W" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

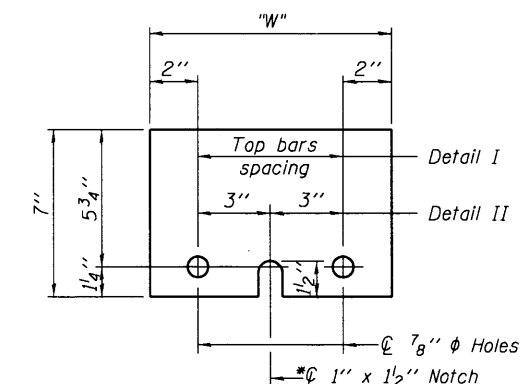
- \*\*\* Dimension shown is minimum required embedment into concrete. If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.
- \*\*\*\* If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.



DETAIL I



DETAIL II



### STEEL RETAINER $\bar{P}$ 1" x 7" x "W"

\* Required only with Detail II

### RETAINER ASSEMBLY

\*\* Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

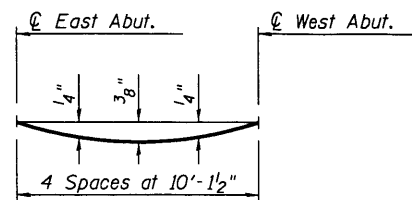
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R-27

1-12-15

FILE NAME = 0610093-76A23-004-Temp Concrete Barrier.dgn	USER NAME = mjreker	DESIGNED - SDG	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION STRUCTURE NO. 061-0093	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		CHECKED - JMT	REVISED -			327	15-BR	MARION	57	33
PLOT SCALE =		DRAWN - SDG	REVISED -							
PLOT DATE = 10/1/2015		CHECKED - SLC	REVISED -							
SHEET NO. 4 OF 16 SHEETS						ILLINOIS FED. AID PROJECT CONTRACT NO. 76A23				



**DEAD LOAD DEFLECTION DIAGRAM**  
(Includes weight of concrete, excluding beams).

**Note:**  
The above deflections are not to be used in the field if the engineer is working from the grade elevations adjusted for dead load deflections as shown below.

The Contractor shall make allowance for the deflection of forms, shrinkage and settlement of falsework, in addition to allowance for dead load deflection.

**FACE OF SOUTH PARAPET**

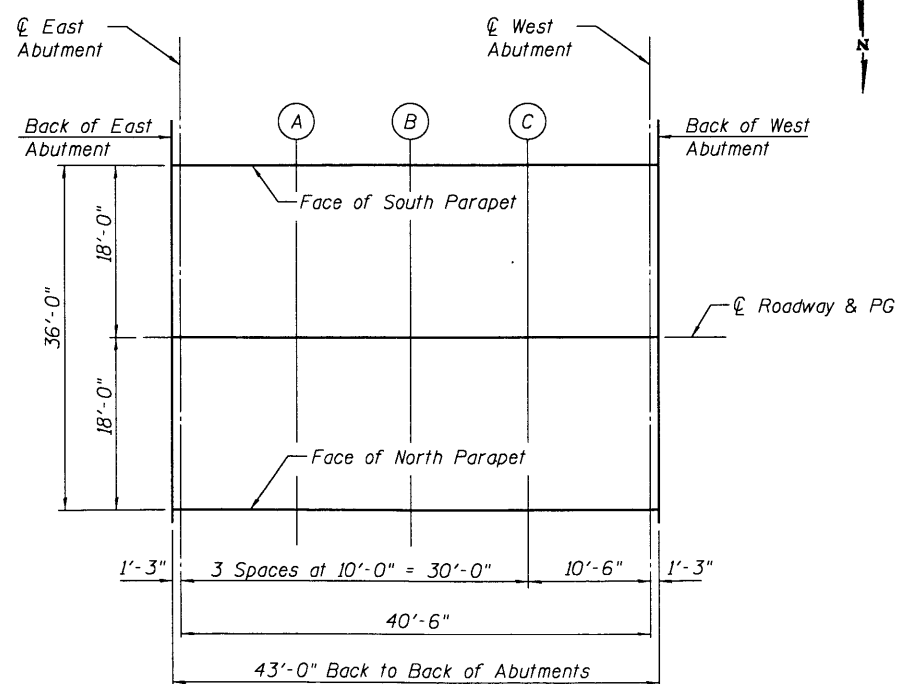
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of E. Abut.	1100+39.50	-18.00	517.85	517.85
CL E. Abut.	1100+40.75	-18.00	517.85	517.85
A	1100+50.75	-18.00	517.85	517.87
B	1100+60.75	-18.00	517.85	517.89
C	1100+70.75	-18.00	517.85	517.87
CL W. Abut	1100+81.25	-18.00	517.85	517.85
Back of W. Abut.	1100+82.50	-18.00	517.85	517.85

**PG & STAGE CONSTRUCTION JOINT**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of E. Abut.	1100+39.50	0.00	518.16	518.16
CL E. Abut.	1100+40.75	0.00	518.17	518.17
A	1100+50.75	0.00	518.17	518.19
B	1100+60.75	0.00	518.17	518.20
C	1100+70.75	0.00	518.17	518.19
CL W. Abut	1100+81.25	0.00	518.17	518.17
Back of W. Abut.	1100+82.50	0.00	518.16	518.16

**FACE OF NORTH PARAPET**

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Back of E. Abut.	1100+39.50	18.00	517.85	517.85
CL E. Abut.	1100+40.75	18.00	517.85	517.85
A	1100+50.75	18.00	517.85	517.87
B	1100+60.75	18.00	517.85	517.89
C	1100+70.75	18.00	517.85	517.87
CL W. Abut	1100+81.25	18.00	517.85	517.85
Back of W. Abut.	1100+82.50	18.00	517.85	517.85



**PLAN**

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FACE OF SOUTH PARAPET - EAST APPROACH

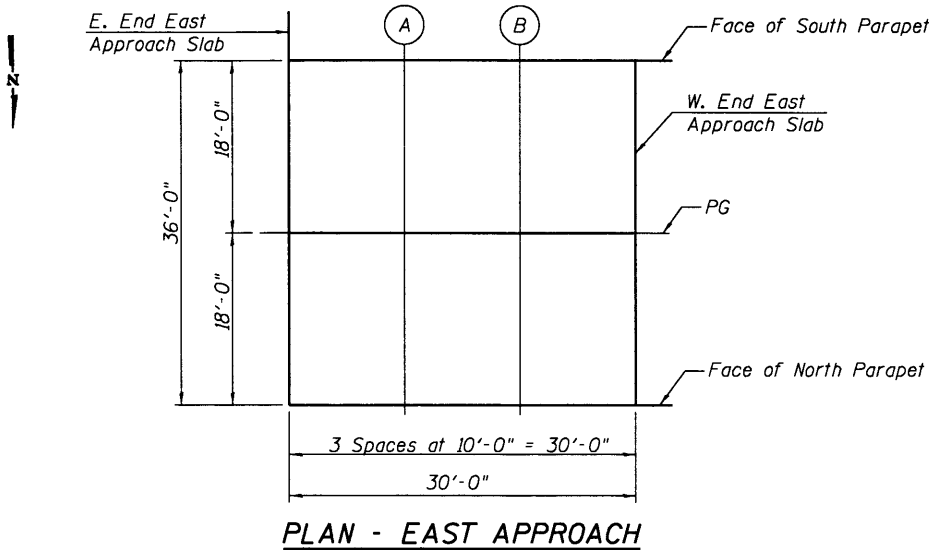
Location	Station	Offset	Theoretical Grade Elevations
E. End East Appr. Slab	1100+10.00	-18.00	517.82
A	1100+20.00	-18.00	517.83
B	1100+30.00	-18.00	517.84
W. End East Appr. Slab	1100+40.00	-18.00	517.85

PG & STAGE CONSTRUCTION JOINT - EAST APPROACH

Location	Station	Offset	Theoretical Grade Elevations
E. End East Appr. Slab	1100+10.00	0.00	518.14
A	1100+20.00	0.00	518.15
B	1100+30.00	0.00	518.16
W. End East Appr. Slab	1100+40.00	0.00	518.17

FACE OF NORTH PARAPET - EAST APPROACH

Location	Station	Offset	Theoretical Grade Elevations
E. End East Appr. Slab	1100+10.00	18.00	517.82
A	1100+20.00	18.00	517.83
B	1100+30.00	18.00	517.84
W. End East Appr. Slab	1100+40.00	18.00	517.85



FACE OF SOUTH PARAPET - WEST APPROACH

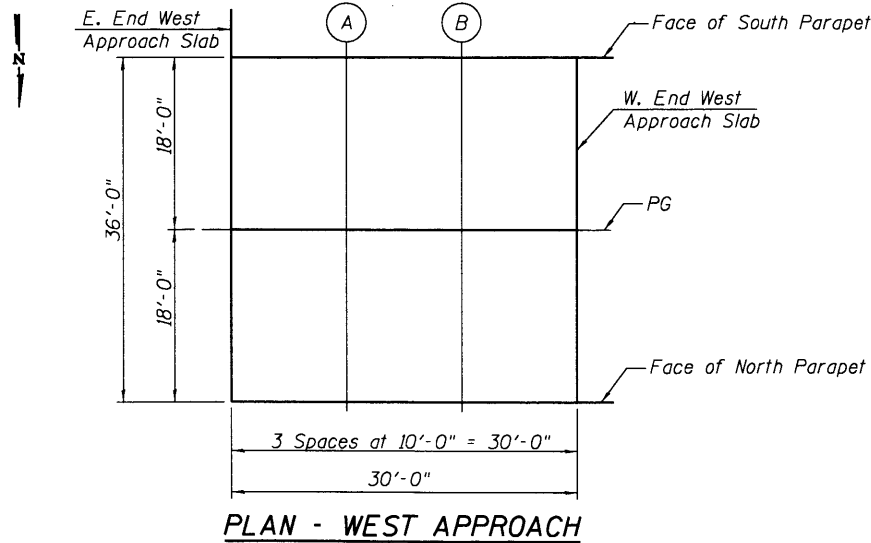
Location	Station	Offset	Theoretical Grade Elevations
E. End West Appr. Slab	1100+82.00	-18.00	517.85
A	1100+92.00	-18.00	517.84
B	1101+02.00	-18.00	517.83
W. End West Appr. Slab	1101+12.00	-18.00	517.82

PG & STAGE CONSTRUCTION JOINT - WEST APPROACH

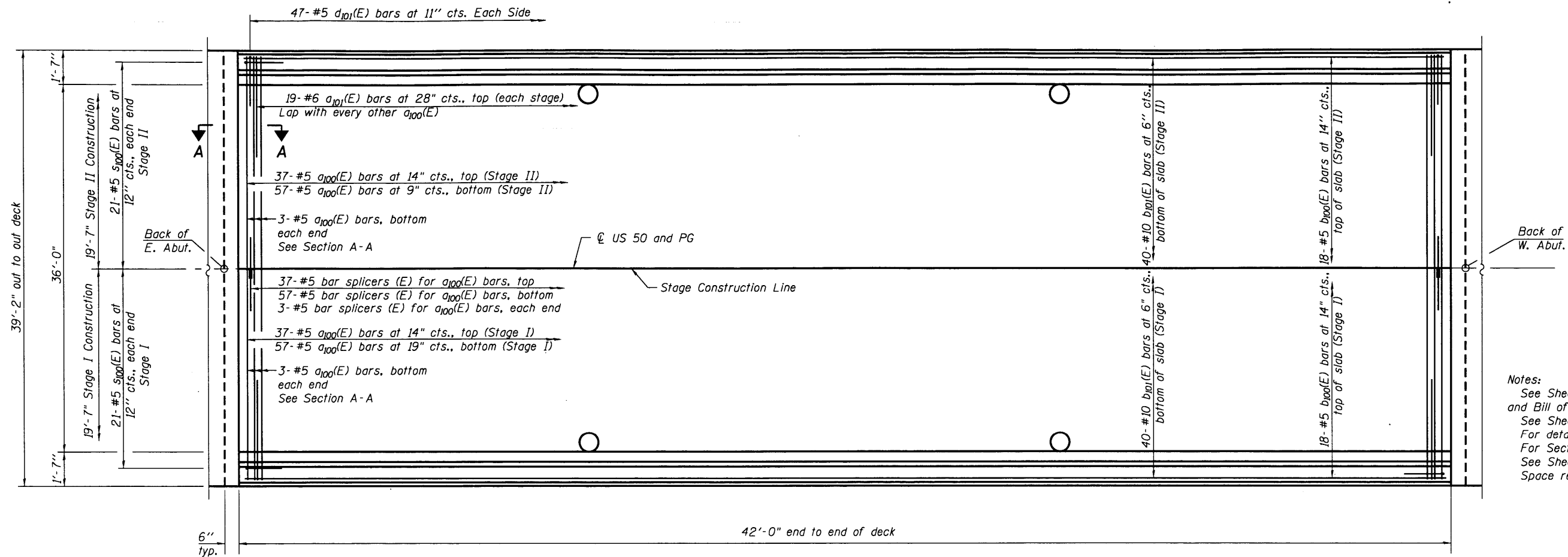
Location	Station	Offset	Theoretical Grade Elevations
E. End West Appr. Slab	1100+82.00	0.00	518.17
A	1100+92.00	0.00	518.16
B	1101+02.00	0.00	518.15
W. End West Appr. Slab	1101+12.00	0.00	518.14

FACE OF NORTH PARAPET - WEST APPROACH

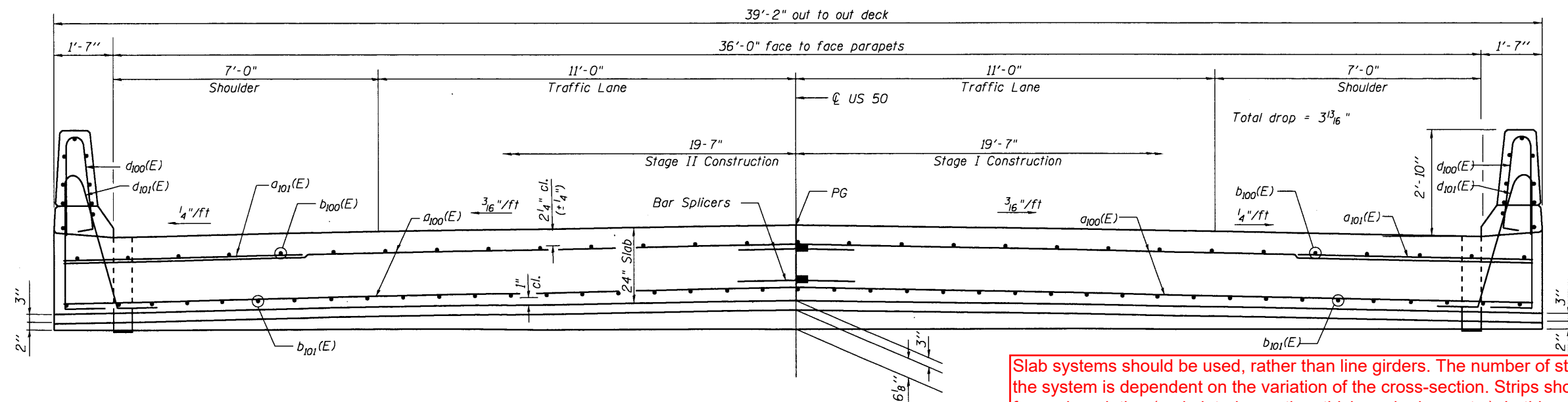
Location	Station	Offset	Theoretical Grade Elevations
E. End West Appr. Slab	1100+82.00	18.00	517.85
A	1100+92.00	18.00	517.84
B	1101+02.00	18.00	517.83
W. End West Appr. Slab	1101+12.00	18.00	517.82



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PLAN



CROSS SECTION  
(Looking West)

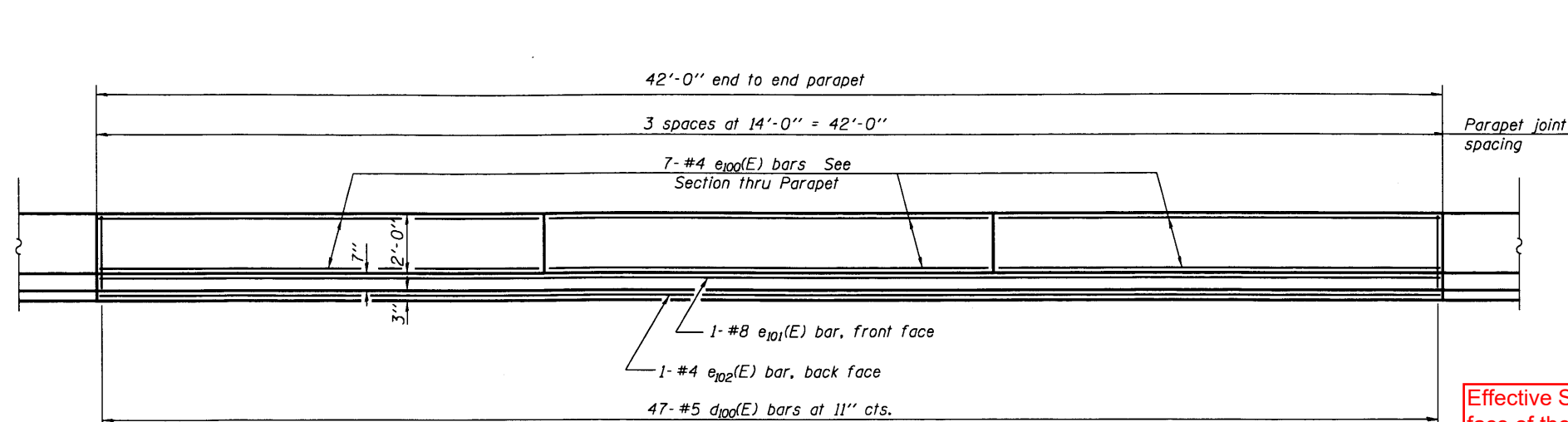
\* Slab strip(s) that controls the overall load rating should have "X" as a suffix in the Member Name and both the "Existing" and "Current" boxes should be checked in the Member window. Non-controlling slab strips should only have the "Current" box checked.

Slab systems should be used, rather than line girders. The number of strips defined in the system is dependent on the variation of the cross-section. Strips should be defined for each variation (main interior portion, thickened edges, etc.). In this case, the cross-section is uniform, so the system was defined with two equal strips. Note that if the slab contains a thickened edge beam with shear stirrup reinforcement, a separate line girder definition must be created since the slab system definition does not allow for shear stirrups.

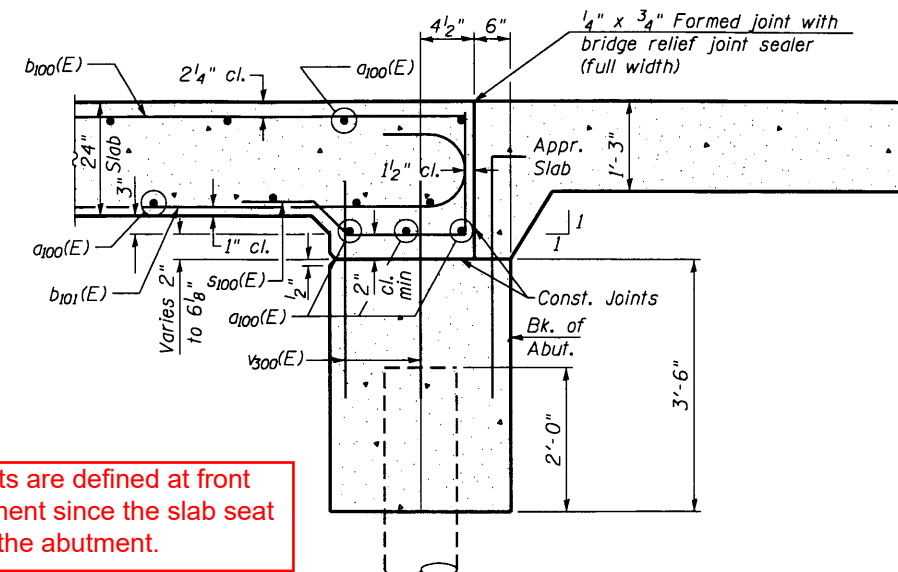
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FILE NAME = 0610093-76A23-007-Superstructure.dgn	USER NAME = mjrocker	DESIGNED - SDG	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	SUPERSTRUCTURE STRUCTURE NO. 061-0093	SHEET NO. 7 OF 16 SHEETS	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		CHECKED - JMT	REVISED -				327	15-BR	MARION	57	36
	PLOT SCALE =	DRAWN - SDG	REVISED -				CONTRACT NO. 76A23				
	PLOT DATE = 10/1/2015	CHECKED - JMT	REVISED -				ILLINOIS FED. AID PROJECT				



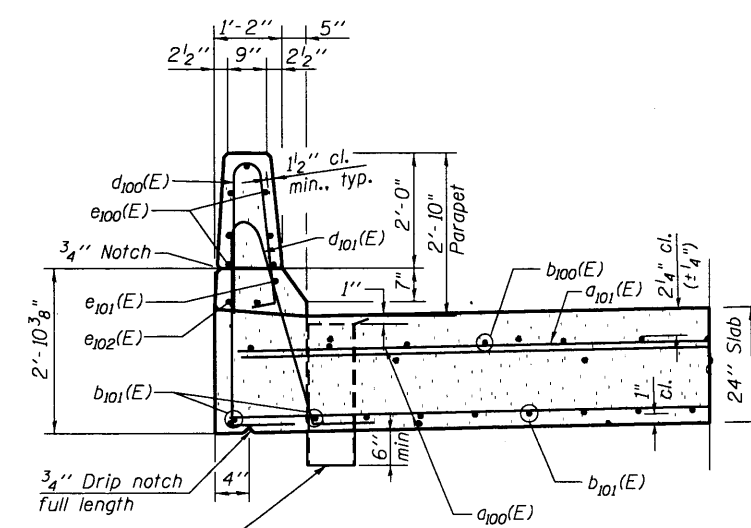


**INSIDE ELEVATION OF PARAPET**  
(Looking North at North parapet)

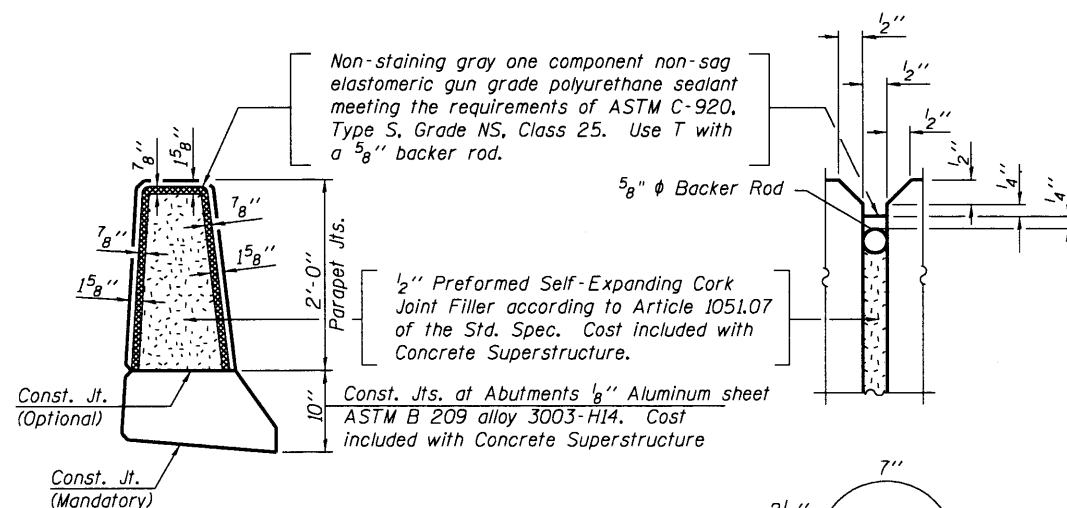


**SECTION A-A**

Effective Supports are defined at front face of the abutment since the slab seat rests directly on the abutment.

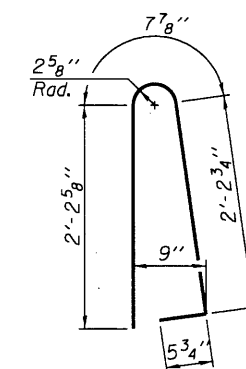


**SECTION THRU PARAPET**

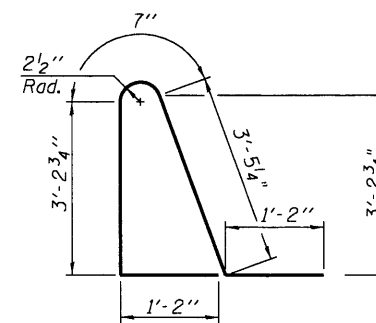


**PARAPET JOINT DETAILS**

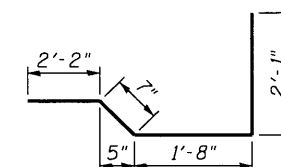
Notes:  
Fiberglass pipe shall conform to ASTM D2996, with short-time rupture strength hoop tensile stress of 30,000 p.s.i. minimum.  
The exterior surfaces of the floor drains shall be coated or pigmented by the manufacturer with a color that matches the concrete.



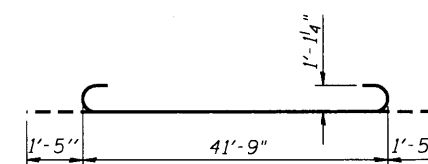
**BAR d100(E)**



**BAR d101(E)**



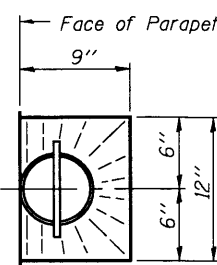
**BAR s100(E)**



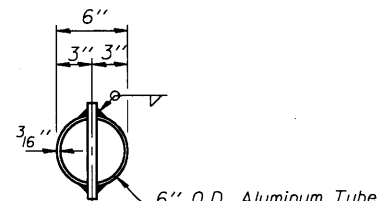
**BAR b101(E)**

**SUPERSTRUCTURE  
BILL OF MATERIAL**

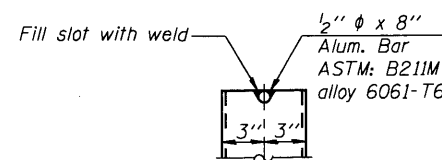
Bar	No.	Size	Length	Shape
a100 (E)	200	#5	19'-4"	—
a101 (E)	38	#6	6'-6"	—
b100 (E)	36	#5	41'-9"	—
b101 (E)	80	#10	44'-7"	—
d100 (E)	94	#5	5'-7"	—
d101 (E)	94	#5	9'-7"	—
e100 (E)	42	#4	13'-9"	—
e101 (E)	2	#8	41'-9"	—
e102 (E)	2	#4	41'-9"	—
s100 (E)	42	#5	6'-6"	—
Reinforcement Bars, Epoxy Coated		Pound	23,760	
Concrete Superstructure		Cu. Yds.	135.1	
Bar Splicers		Each	100	



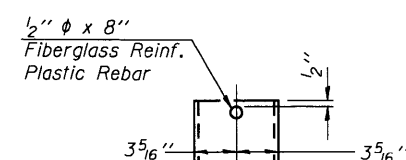
**TOP PLAN**



**TOP PLAN**  
(Showing Aluminum Tube)



**ALUMINUM  
TUBE**



**FIBERGLASS  
PIPE**

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DEPARTMENT OF TRANSPORTATION**

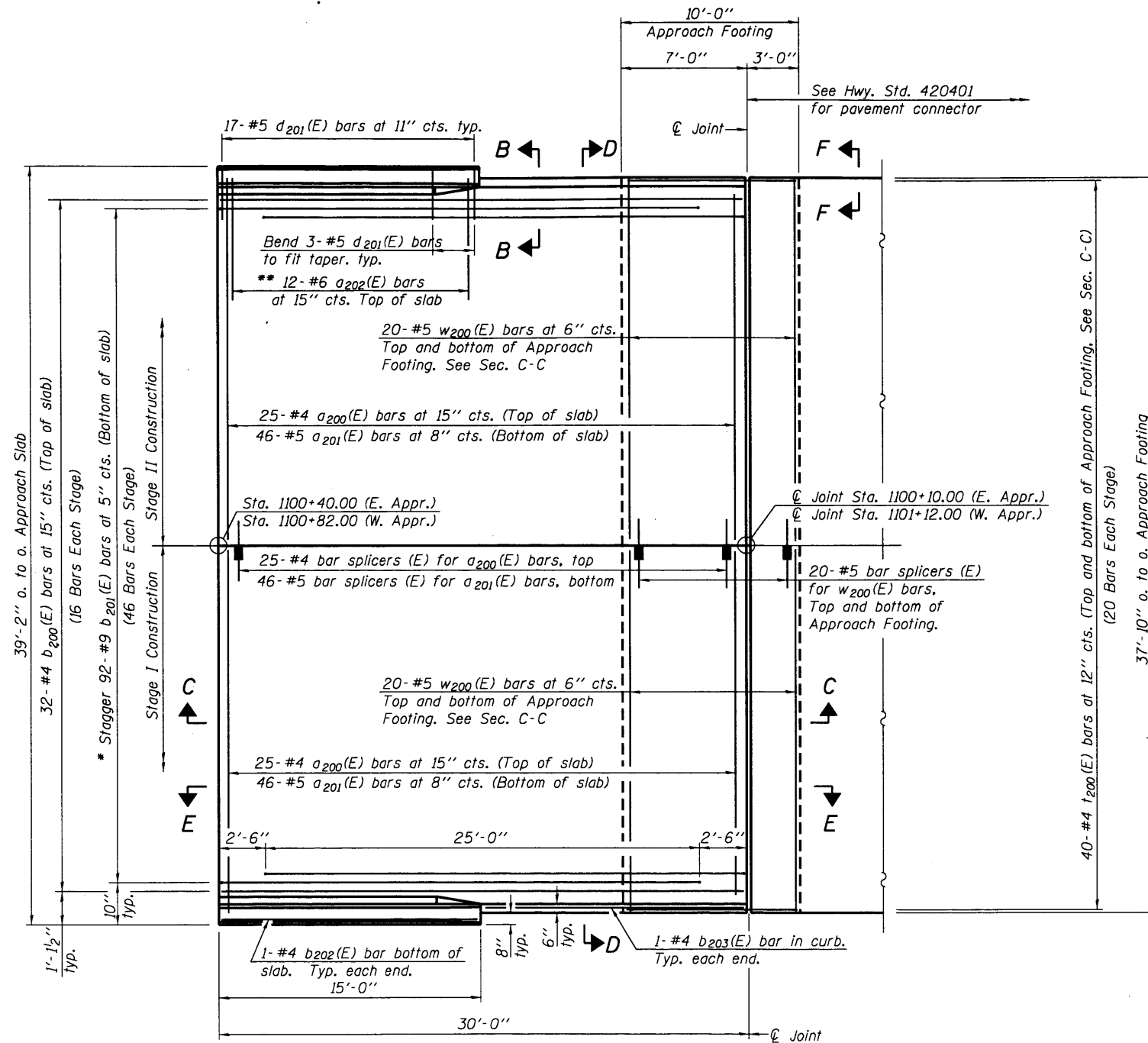
**SUPERSTRUCTURE DETAILS  
STRUCTURE NO. 061-0093**

SHEET NO. 8 OF 16 SHEETS

F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
327	15-BR	MARION	57	37
CONTRACT NO. 76A23				
ILLINOIS FED. AID PROJECT				

FILE NAME = 0610093-76A23-008-superstr details.dgn	USER NAME = mjreker	DESIGNED - SDG	REVISED -
		CHECKED - JMT	REVISED -
		DRAWN - SDG	REVISED -
		CHECKED - JMT	REVISED -

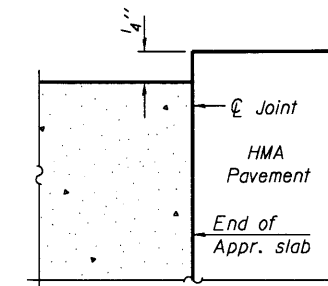
Notes:  
See sheet 10 of 16 for Sections C-C & D-D and View E-E.  
a<sub>200</sub>(E) and a<sub>201</sub>(E) bar spacings measured along  $\phi$  Rdwy.



### PLAN

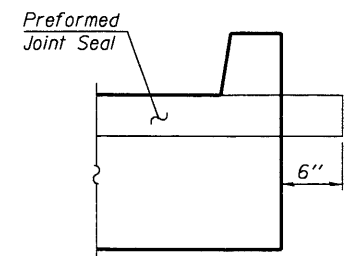
(West approach shown)

- \* Tilt #9 b<sub>201</sub>(E) bars as required to maintain clearance.
- \*\* Space between a<sub>200</sub>(E) bars, typ. ea. parapet.

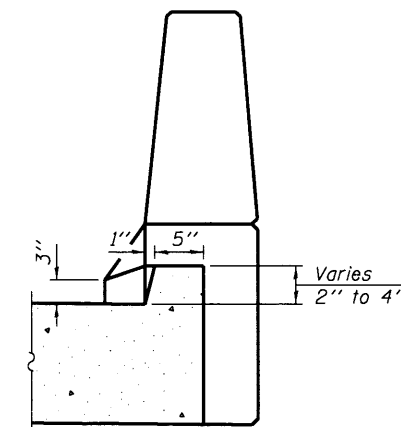


### FLEXIBLE PAVEMENT

### DETAIL A



### VIEW F-F



### VIEW B-B

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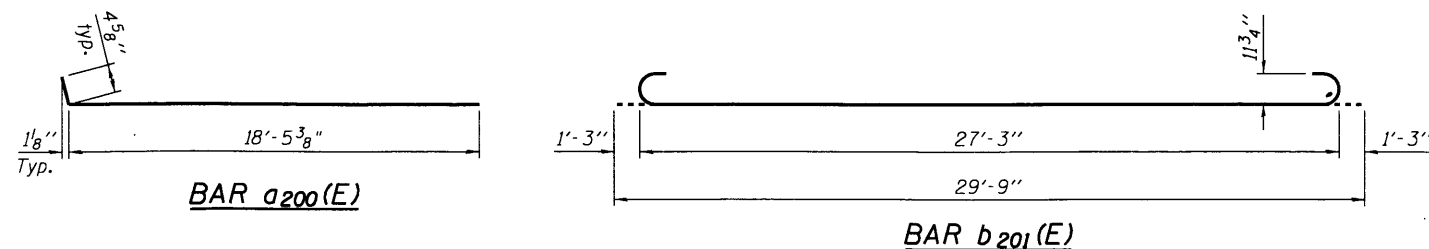
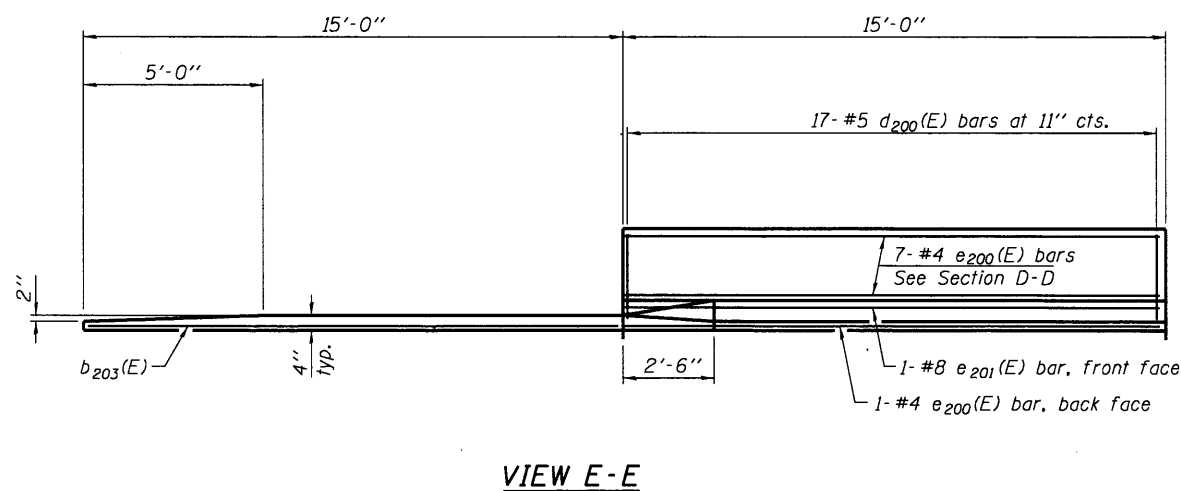
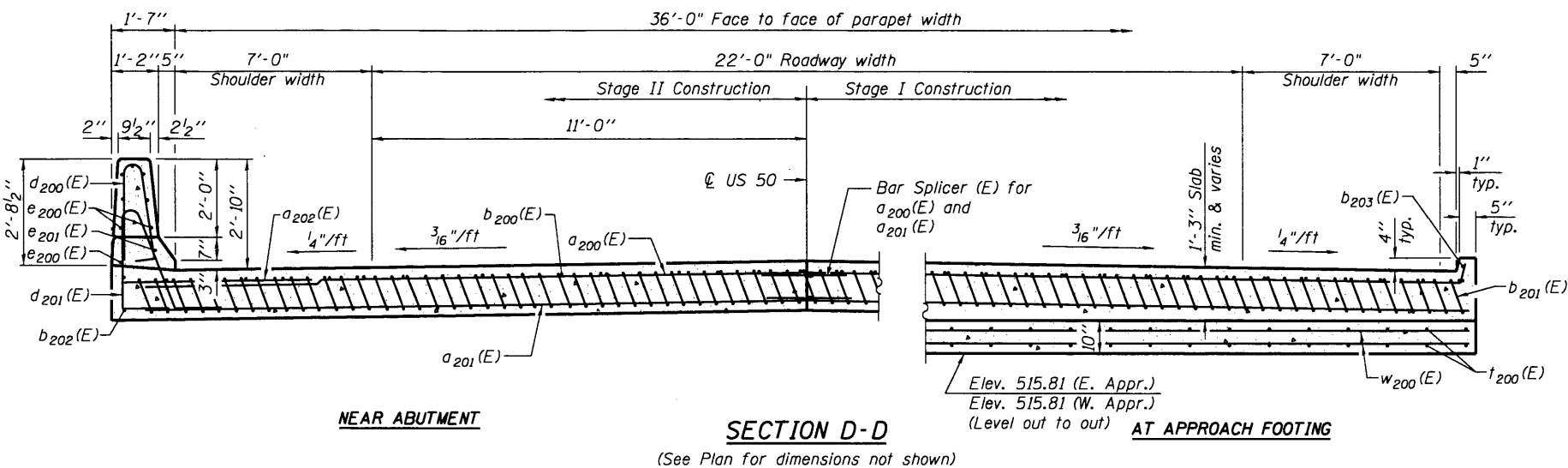
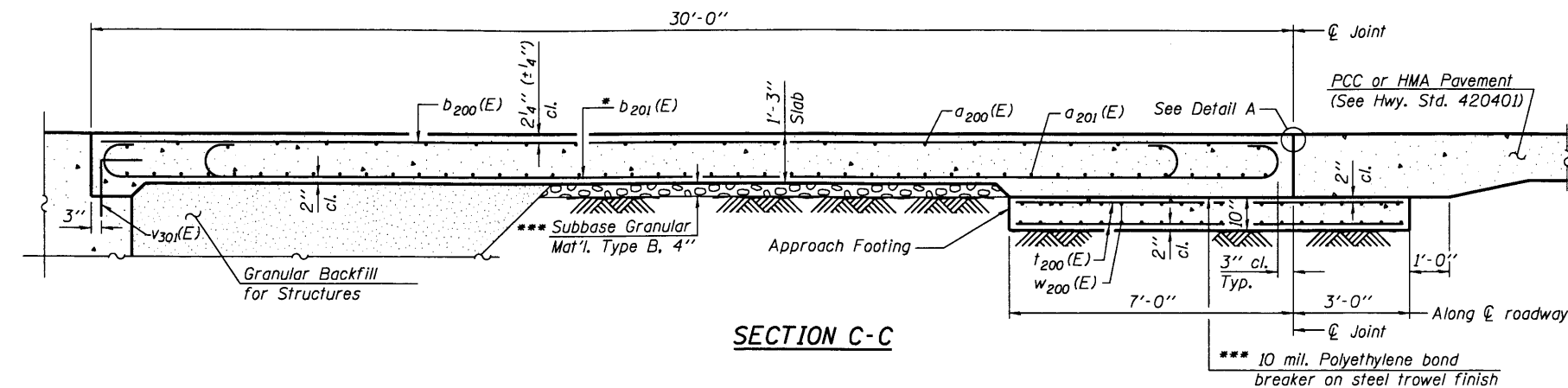
STATE OF ILLINOIS  
DEPARTMENT OF TRANSPORTATION

BRIDGE APPROACH SLAB DETAILS I  
STRUCTURE NO. 061-0093

SHEET NO. 9 OF 16 SHEETS

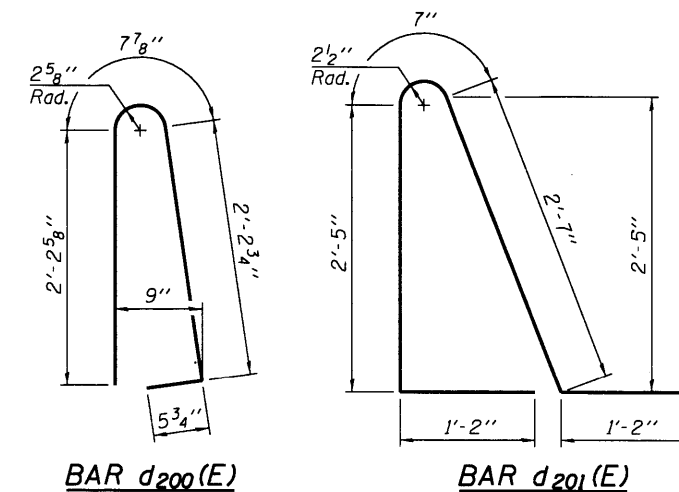
F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
327	15-BR	MARION	57	38
CONTRACT NO. 76A23				
ILLINOIS FED. AID PROJECT				

FILE NAME = 0610093-76A23-009-Appr slab details.dgn	USER NAME = mjreker	DESIGNED - SDG	REVISED -
		CHECKED - JMT	REVISED -
		DRAWN - SDG	REVISED -
		CHECKED - SLC	REVISED -
PLOT SCALE =			
PLOT DATE = 10/1/2015			



# Notes:

See sheet 9 of 16 for Detail A and View B-B.  
Approach slab and parapet concrete shall be paid for as Concrete Superstructure.  
Approach footing concrete shall be paid for as Concrete Structures.  
Reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.  
For v301(E) bar details, see sheet 11 of 16.  
The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.  
For bar splicer details, see sheet 12 of 16.  
Cost of excavation for approach footing included with Concrete Structures.  
For Granular Backfill for Structures and drainage treatment details, see sheet 12 of 16.  
For additional parapet details, see sheet 8 of 16.



\* Tilt #9 b201(E) bars as required to maintain clearance.

\*\*\* Cost included with Concrete Superstructure.

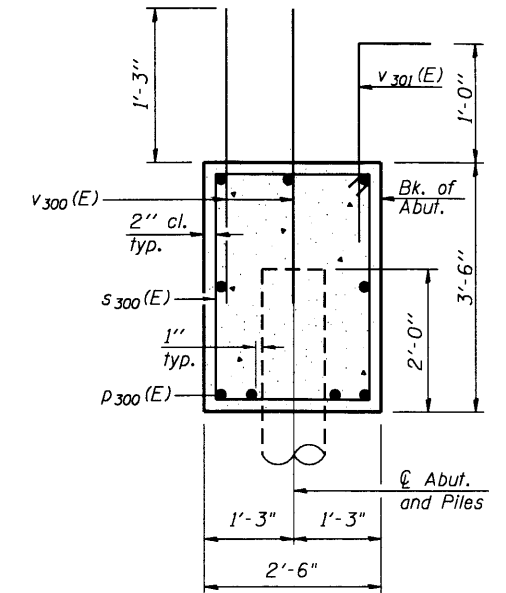
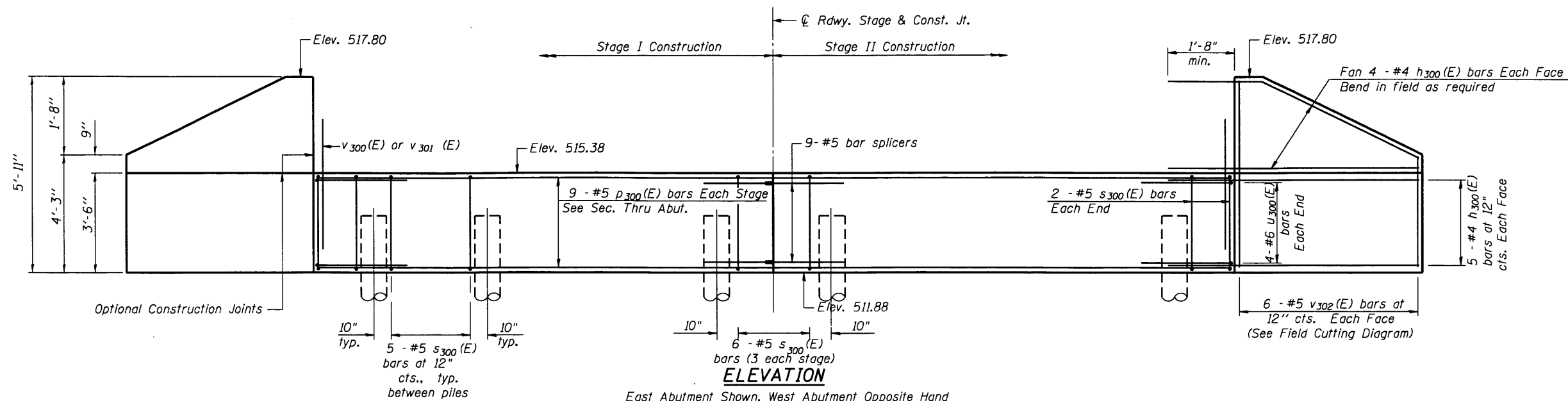
## TWO APPROACHES BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a200 (E)	100	#4	18'-10"	
a201 (E)	184	#5	18'-8"	
a202 (E)	24	#6	6'-6"	
b200 (E)	64	#4	29'-8"	
b201 (E)	184	#9	29'-9"	
b202 (E)	4	#4	14'-8"	
b203 (E)	4	#4	14'-9"	
d200 (E)	68	#5	5'-7"	
d201 (E)	68	#5	7'-11"	
e200 (E)	32	#4	14'-8"	
e201 (E)	4	#8	14'-8"	
t200 (E)	80	#4	9'-8"	
w200 (E)	160	#5	18'-8"	
Concrete Superstructure		Cu. Yd.	135.4	
Concrete Structures		Cu. Yd.	23.4	
Reinforcement Bars, Epoxy Coated		Pound	30,100	
Bar Splicers		Each	222	

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FILE NAME = 0610093-76A23-010-Appr slab details2.dgn	USER NAME = mjreker	DESIGNED - SDG	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	BRIDGE APPROACH SLAB DETAILS II STRUCTURE NO. 061-0093	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		CHECKED - JMT	REVISED -			327	15-BR	MARION	57	39
	PLOT SCALE =	DRAWN - SDG	REVISED -			CONTRACT NO. 76A23				
	PLOT DATE = 10/1/2015	CHECKED - SLC	REVISED -			ILLINOIS FED. AID PROJECT				
SHEET NO. 10 OF 16 SHEETS										



SEC. THRU ABUT.

BILL OF MATERIAL  
TWO ABUTMENTS

Bar	No.	Size	Length	Shape
h <sub>300</sub> (E)	72	#4	6'-3"	
p <sub>300</sub> (E)	36	#5	19'-3"	
s <sub>300</sub> (E)	80	#5	11'-7"	
u <sub>300</sub> (E)	16	#6	11'-0"	
v <sub>300</sub> (E)	168	#5	3'-5"	
v <sub>301</sub> (E)	84	#5	2'-8"	
v <sub>302</sub> (E)	24	#5	9'-6"	
Structure Excavation		Cu. Yd.	188	
Concrete Structures		Cu. Yd.	28.8	
Reinforcement Bars, Epoxy Coated		Pound	3,330	
Furnishing Metal Shell Piles 14" x 0.25"		Foot	259	
Driving Piles		Foot	259	
Test Pile Metal Shells		Each	1	
Bar Splicers		Each	18	

For details of Bar Splicers, see sheet 12 of 16.  
For details of piles, see sheet 13 of 16.

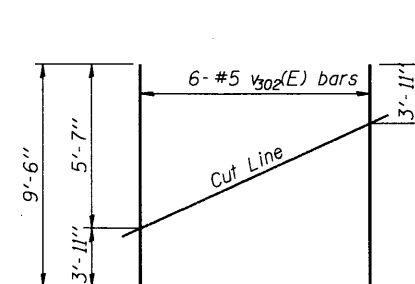
PILE DATA - W Abut.

Type: Metal Shell - 14"  $\phi$  x 0.25 in. walls  
Nominal Required Bearing: 222 kips  
Factored Resistance Available: 122 kips  
Est. Length: 21 ft.  
No. Production Piles: 7  
No. Test Piles: 1

PILE DATA - E Abut.

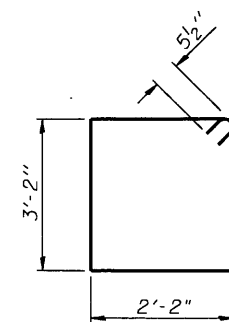
Type: Metal Shell - 14"  $\phi$  x 0.25 in. walls  
Nominal Required Bearing: 313 kips  
Factored Resistance Available: 172 kips  
Est. Length: 14 ft.  
No. Production Piles: 8  
No. Test Piles: 0

PLAN  
East Abutment Shown, West Abutment Opposite Hand

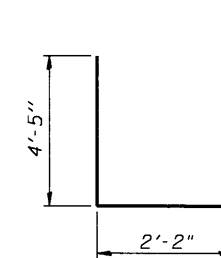


FIELD CUTTING DIAGRAM

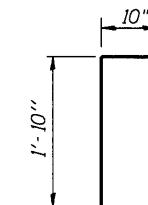
Order v<sub>302</sub>(E) full length. Cut as shown and use remainder of bars in opposite face.



BAR s<sub>300</sub>(E)



BAR u<sub>300</sub>(E)



BAR v<sub>301</sub>(E)

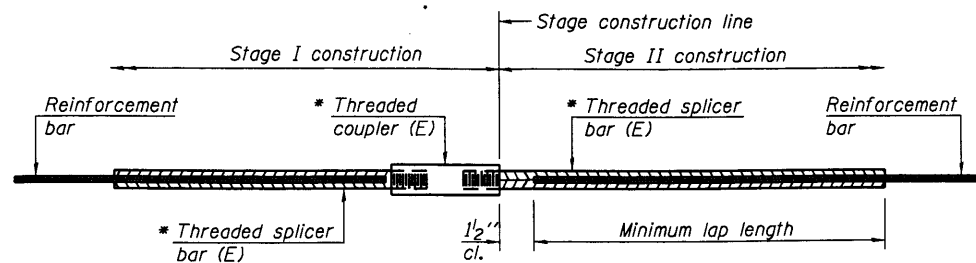
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AI-0

7-1-10

FILE NAME = 0610093-76A23-011-AbutDetails.dgn	USER NAME = mjraker	DESIGNED - SDG	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	ABUTMENT DETAILS STRUCTURE NO. 061-0093	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		CHECKED - JMT	REVISED -			327	15-BR	MARION	57	40
	PLOT SCALE *	DRAWN - SDG	REVISED -			CONTRACT NO. 76A23				
	PLOT DATE = 10/1/2015	CHECKED - JMT	REVISED -			ILLINOIS FED. AID PROJECT				



### STANDARD BAR SPLICER ASSEMBLY

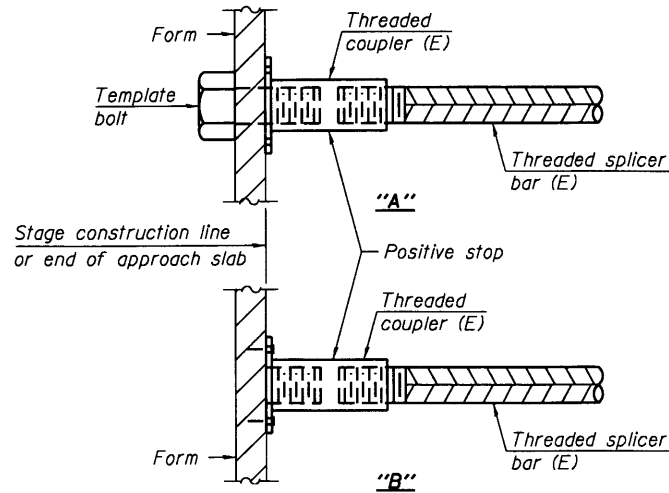
Minimum Lap Lengths						
Bar size to be spliced	Table 1	Table 2	Table 3	Table 4	Table 5	Table 6
3, 4	1'-5"	1'-11"	2'-1"	2'-4"	2'-7"	2'-11"
5	1'-9"	2'-5"	2'-7"	2'-11"	3'-3"	3'-8"
6	2'-1"	2'-11"	3'-1"	3'-6"	3'-10"	4'-5"
7	2'-9"	3'-10"	4'-2"	4'-8"	5'-2"	5'-10"
8	3'-8"	5'-1"	5'-5"	6'-2"	6'-9"	7'-8"
9	4'-7"	6'-5"	6'-10"	7'-9"	8'-7"	9'-8"

Table 1: Black bar, 0.8 Class C  
 Table 2: Black bar, Top bar lap, 0.8 Class C  
 Table 3: Epoxy bar, 0.8 Class C  
 Table 4: Epoxy bar, Top bar lap, 0.8 Class C  
 Table 5: Epoxy bar, Class C  
 Table 6: Epoxy bar, Top bar top, Class C

Threaded splicer bar length = min. lap length + 1 1/2" + thread length

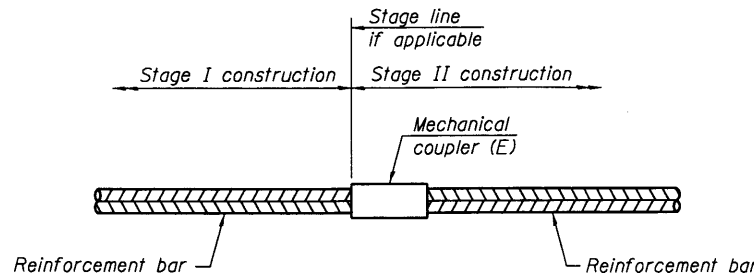
\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar size	No. assemblies required	Table for minimum lap length
Superstructure	#5	100	Table 4
Approach	#5	172	Table 3
Approach	#4	50	Table 3
Abutment	#5	18	Table 3



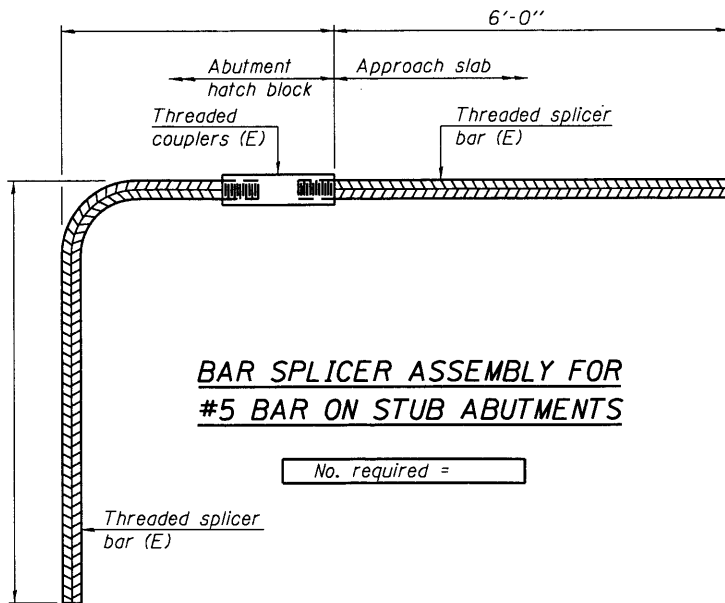
### INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt.  
 "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.  
 (E) : Indicates epoxy coating.



### STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



### BAR SPLICER ASSEMBLY FOR #5 BAR ON STUB ABUTMENTS

No. required =

### NOTES

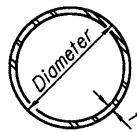
Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.  
 All reinforcement shall be lapped and tied to the splicer bars.  
 Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.  
 See approved list of bar splicer assemblies and mechanical splicers for alternatives.

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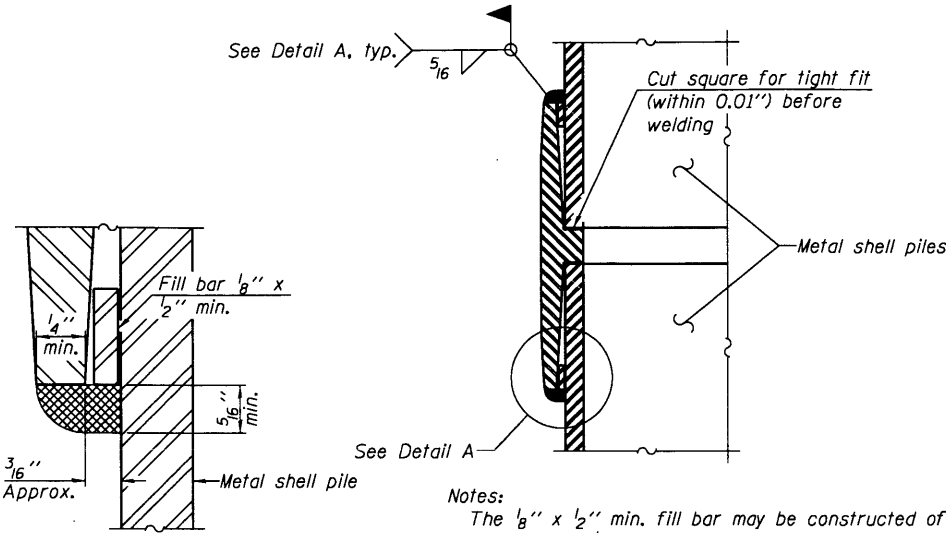
BSD-1 8-31-12

FILE NAME = 0610093-76A23-012-Bar Splicer details.dgn	USER NAME = mjreker	DESIGNED - SDG	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS STRUCTURE NO. 061-0093	SHEET NO. 12 OF 16 SHEETS	F.A.P. RTE. 327	SECTION 15-BR	COUNTY MARION	TOTAL SHEETS 57	SHEET NO. 41
	PLOT SCALE =	CHECKED - JMT	REVISED -								
	PLOT DATE = 10/1/2015	DRAWN - SDG	REVISED -								
		CHECKED - SLC	REVISED -								



METAL SHELL PILE TABLE

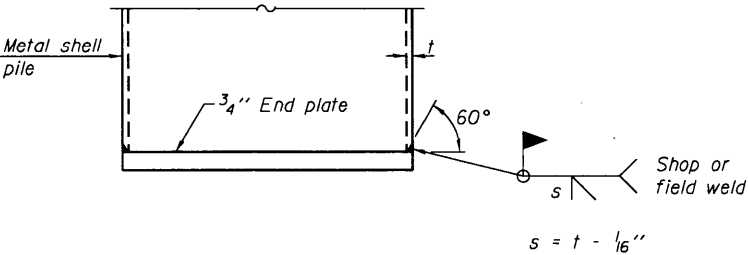
Designation and outside diameter	Wall thickness t	Weight per foot (Lbs./ft.)	Inside volume (yd. <sup>3</sup> /ft.)
PP12	0.179"	22.60	0.0274
PP12	0.250"	31.37	0.0267
PP14	0.250"	36.71	0.0368
PP14	0.312"	45.61	0.0361



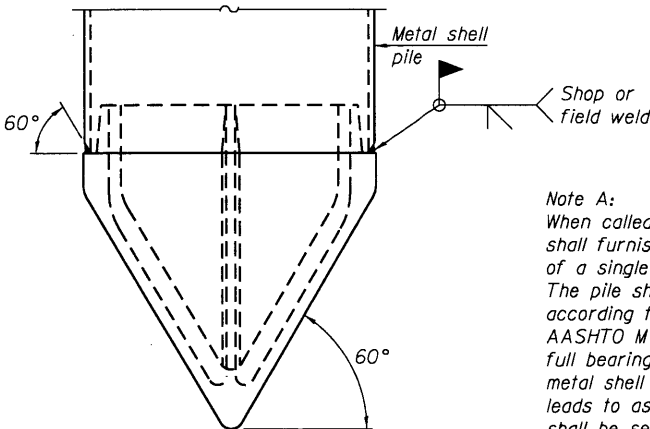
DETAIL A

Notes:  
The 1/8" x 1/2" min. fill bar may be constructed of 2 bars with a 1/8" max. gap between them.  
Pile segments shall be driven to solid contact with splicer before welding.

WELDED COMMERCIAL SPLICE



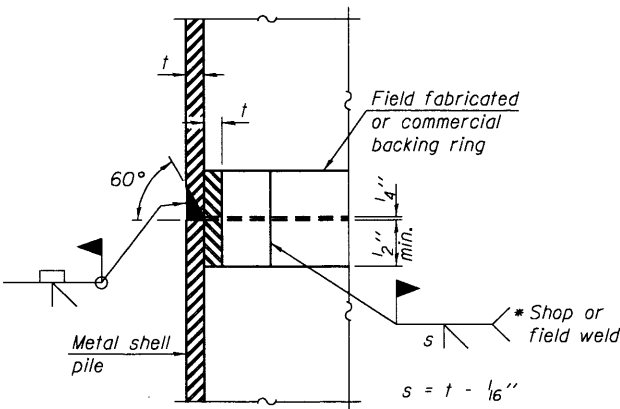
END PLATE ATTACHMENT



Note A:  
When called for on the plans, the Contractor shall furnish metal shell pile shoes consisting of a single piece conical pile point as shown. The pile shoes shall be cast in one piece steel according to either ASTM A 148 Grade 90-60 or AASHTO M 103 Grade 65-35 and shall provide full bearing over the full circumference of the metal shell pile. The pile shoe shall have tapered leads to assure proper alignment and fitting and shall be secured to the pile with a circumferential weld.

METAL SHELL PILE SHOE ATTACHMENT

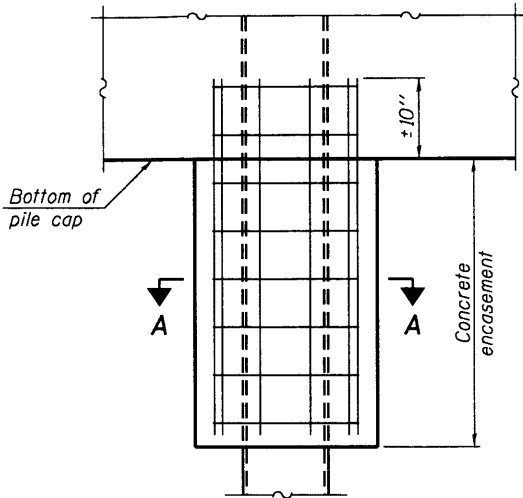
(See Note A)



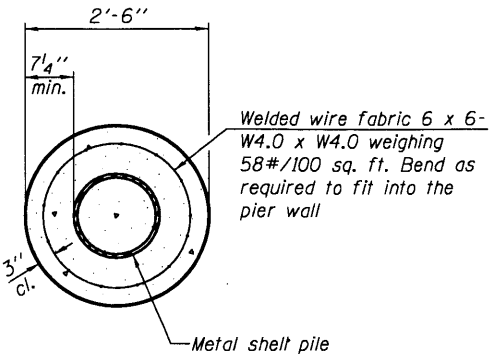
COMPLETE PENETRATION WELD SPLICE

\* Field fabricated backing ring may be made from pile shell by removing segment to allow reducing circumference and vertically rejoin with partial joint penetration weld.

Note:  
The metal shell piles shall be according to ASTM A 252 Grade 3.



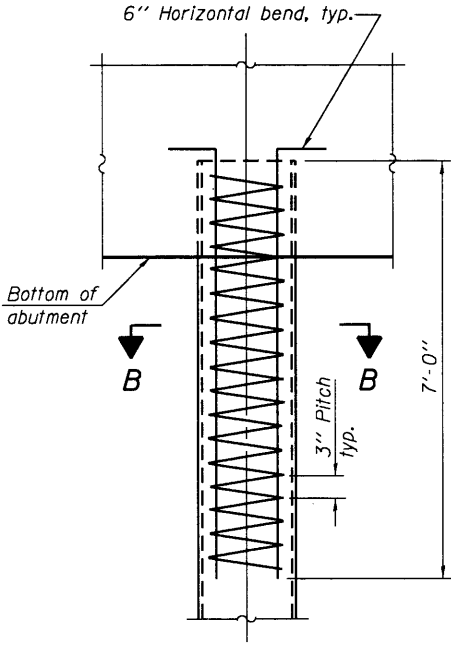
ELEVATION



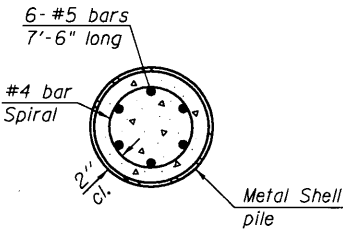
SECTION A-A

Note:  
Forms for encasement may be omitted when soil conditions permit.

CONCRETE ENCASEMENT AT PIERS



ELEVATION



SECTION B-B

METAL SHELL REINFORCEMENT AT ABUTMENTS

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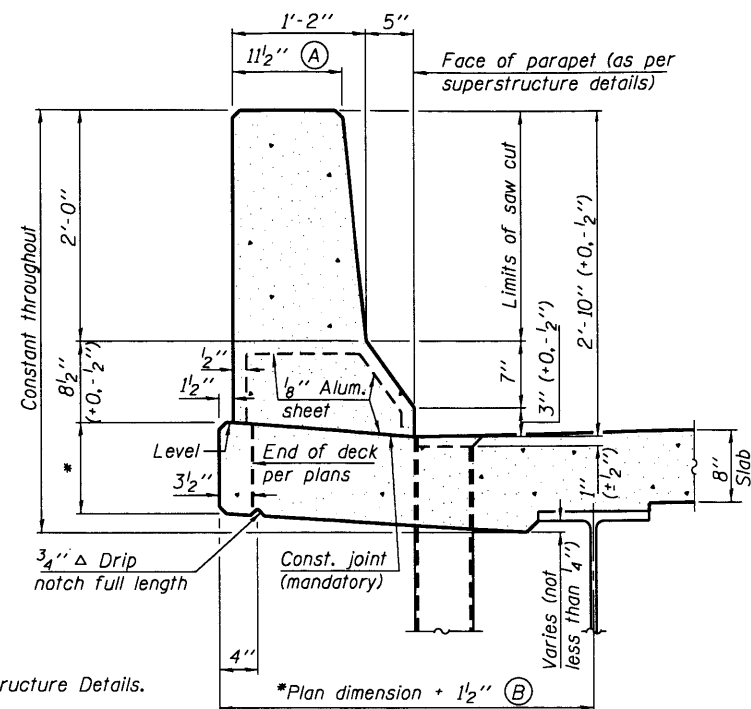
F-MS

1-27-12

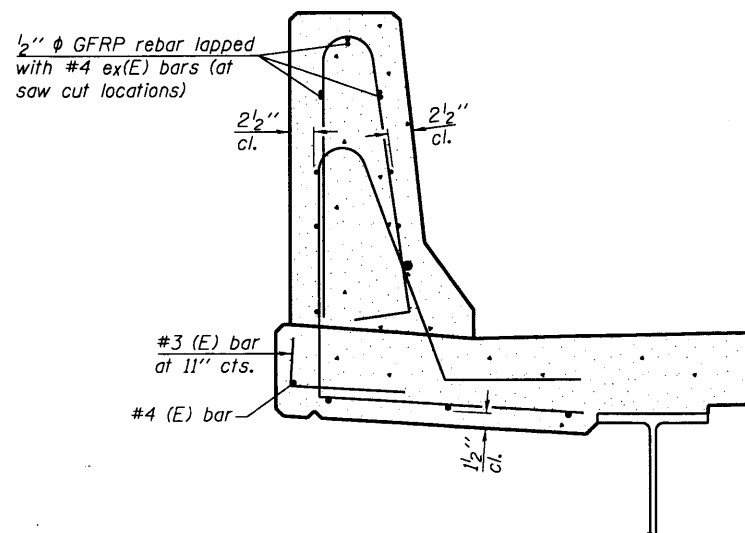
FILE NAME = 0610093-76A23-013- Piles Details.dgn	USER NAME = mjreker	DESIGNED - SDG	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	METAL SHELL PILE DETAILS STRUCTURE NO. 061-0093 SHEET NO. 13 OF 16 SHEETS	F.A.P. RTE. 327	SECTION 15-BR	COUNTY MARION	TOTAL SHEETS 57	SHEET NO. 42
	PLOT SCALE =	CHECKED - JMT	REVISED -							
	PLOT DATE = 10/1/2015	DRAWN - SDG	REVISED -							
		CHECKED - SLC	REVISED -							

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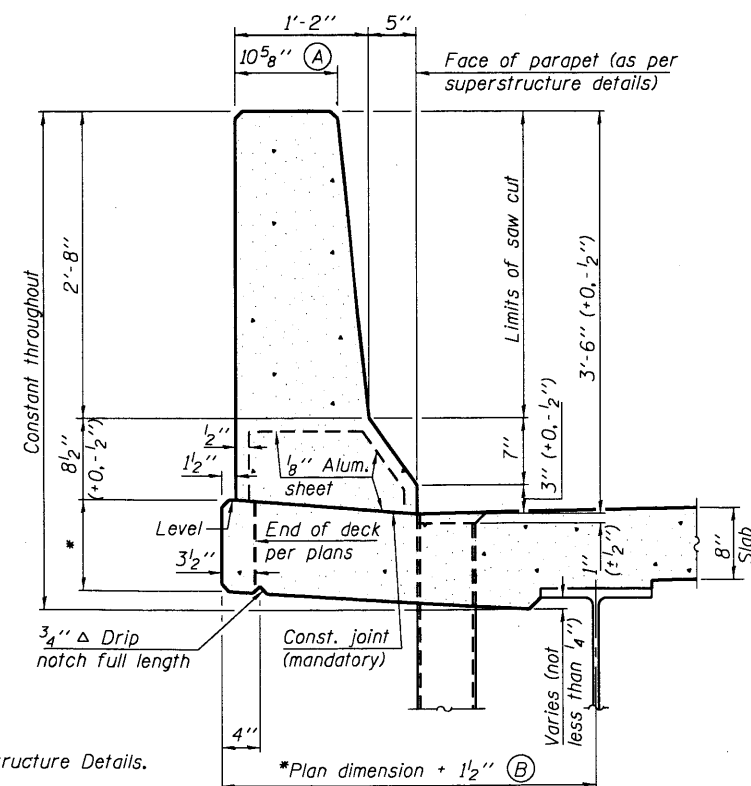


**34" F SHAPE PARAPET SECTION**  
(Showing dimensions)

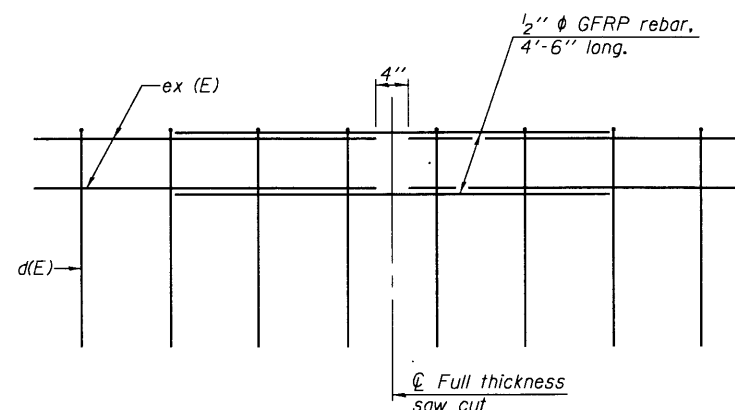


**SECTION**

(34" parapet shown - 42" parapet similar)  
(Showing reinforcement clearances for slip forming and additional reinforcement bars)



**42" F SHAPE PARAPET SECTION**  
(Showing dimensions)

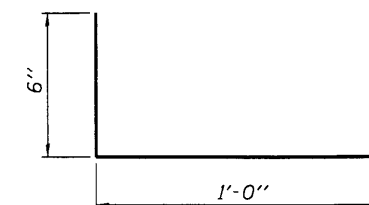


**GFRP REBAR STIFFENING DETAIL**

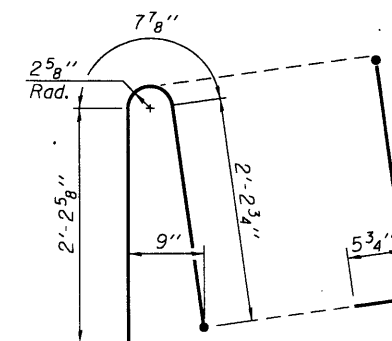
(Place as shown in parapet section at each parapet joint location.)

**GENERAL NOTES**

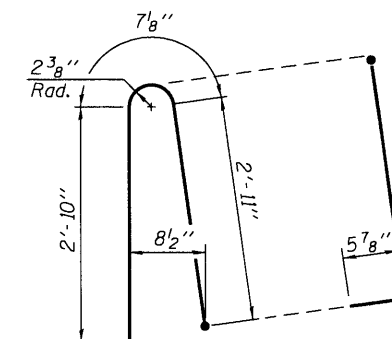
All dimensions shall remain the same as shown on superstructure details, except dimensions A and B which are to be revised as shown to provide additional clearance. Additional concrete needed to revise dimension A and B = 0.0165 cu. yds./ft. for 34" parapet or = 0.0223 cu. yds./ft. for 42" parapet. Place aluminum sheet in curb portion at and near piers. Full thickness saw cut at all joint locations in lieu of cork joint filler. Steel superstructure shown. Other superstructure types similar.



**#3 (E) BAR**



**ALTERNATE BAR d(E)**  
(For 34" parapet when conduit is present)



**ALTERNATE BAR d(E)**  
(For 42" parapet when conduit is present)

\*See Superstructure Details.

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**SFP 34-42**

8-16-12

FILE NAME = 0610093-76A23-013a-SFP-34.dgn	USER NAME = mjreker	DESIGNED - SDG	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	CONCRETE PARAPET SLIPFORMING OPTION STRUCTURE NO. 061-0093	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		CHECKED - JMT	REVISED -			327	15-BR	MARION	57	43
	PLOT SCALE =	DRAWN - SDG	REVISED -							
	PLOT DATE = 10/1/2015	CHECKED - SLC	REVISED -							
	SHEET NO. 14 OF 16 SHEETS					ILLINOIS FED. AID PROJECT				
						CONTRACT NO. 76A23				



## Page 1 of 3

Date 6/21/10

ROUTE FAP 327 DESCRIPTION US 50 over Brubaker Creek Tributary LOGGED BY VPG

SECTION 15BR LOCATION SEC. 9, TWP. 2N, RNG. 3E, 3 PM

COUNTY	Marion	DRILLING METHOD	Hollow Stem Auger	HAMMER TYPE	140# Automatic
--------	--------	-----------------	-------------------	-------------	----------------

STRUCT. NO. 061-0065 (E) /  
061-0093 (P)

BORING NO. <u>1 E Abut</u>	T	W		S	Groundwater Elev.:	T	W		S
Station <u>1100+33</u>	H	S	Qu	T	First Encounter <u>513.5</u> ft	H	S	Qu	T
Offset <u>14.00 ft Right</u>					Upon Completion _____ ft				
Ground Surface Elev. <u>517.5</u> ft	(ft)	(ft)	(ft)	(%)	After _____ Hrs.	(ft)	(ft)	(ft)	(%)

[illegible]

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)  
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)



## Page 2 of 3

Date 6/21/10

ROUTE FAP 327 DESCRIPTION US 50 over Brubaker Creek Tributary LOGGED BY VPG

SECTION 1588 LOCATION SEC. 9, TWP. 2N, RNG. 3E, 3 PM

COUNTY Marion DRILLING METHOD Hollow Stem Auger HAMMER TYPE 140# Automatic

STRUCT. NO. 061-0065 (E) / Station 061-0093 (P)

BORING NO. <u>1 E Abut</u>	T	W	S	Groundwater Elev.: First Encounter <u>513.5</u> ft $\nabla$ Upon Completion _____ ft After _____ Hrs. _____ ft
Station <u>1100+33</u>	H	S	Qu	
Offset <u>14.00ft Right</u>				
Ground Surface Elev. <u>517.5</u> ft	(ft)	(ft)	(%)	

[illegible]

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)  
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

BBS, from 137 (Rev. 8-99)

Page 3 of 3

Date 6/21/10

ROUTE	FAP 327	DESCRIPTION	US 50 over Brubaker Creek Tributary	LOGGED BY	VPG
-------	---------	-------------	-------------------------------------	-----------	-----

SECTION 15BR LOCATION SEC. 9, TWP. 2N, RNG. 3E, 3 PM

COUNTY	Marion	CORING METHOD	R	DEPTH	CORE	S
--------	--------	---------------	---	-------	------	---

STRUCT. NO. 061-0065 (E) / 061-0093 (P) CORING BARREL TYPE & SIZE \_\_\_\_\_

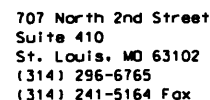
BORING NO. <u>1 E Abut</u>	Top of Rock Elev. <u>469.50</u> ft	P	R	E	D	E	G
Station <u>1100+33</u>	Begin Core Elev. <u>469.50</u> ft	T	E				H
Offset <u>14.00ft Right</u>		H	Y				
Ground Surface Elev. <u>517.5</u> ft		(ft)	(#)	(%)	(%)	(min/ft)	(tsf)

Weathered SHALE	469.50			19.82
				19.41
				15.46
	466.50			32.64
Gray SHALE	465.25			
Core Barrel Clogged - End of Boring				

Color pictures of the cores Yes  
Cores will be stored for examination until Indefinite

The "Strength" column represents the uniaxial compressive strength of the core sample (ASTM D-2938).

BBS, form 138 (Rev. 8-99)



FILE NAME = 0610093-76A23-014-Boring logs 1.dgn	USER NAME = mjreker	DESIGNED - SDG	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	BORING LOGS I STRUCTURE NO. 061-0093	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
		CHECKED - JMT	REVISED -			327	15-BR	MARION	57	44	
	PLOT SCALE =	DRAWN - DMG	REVISED -								
	PLOT DATE = 10/1/2015	CHECKED - SLC	REVISED -								
	SHEET NO. 15 OF 16 SHEETS						ILLINOIS FED. AID PROJECT				
						CONTRACT NO.					



## Page 1 of 2

Date 6/23/10

[illegible]

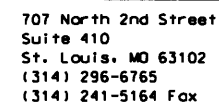
BBS, from 137 (Rev. 8-99)

Page 2 of 2

Date 6/23/10

	DEPTH Feet	GRAIN SIZE Analysis	SOLIDS Content, %	WATER Content, %
Gray Silty LOAM A-4(8) (continued)	22 37	6.11 S/O	21	
	-45			
470.5	3			
Gray Weathered SHALE	7 15	2.24 S/O	15	
	-50			
	8			
464.0	24 74	7.33 S/O	21	
END OF BORING	-55			
	20			

BBS, from 137 (Rev. 8-99)



FILE NAME = 0610093-76A23-015-Boring logs 2.dgn	USER NAME = mjreker	DESIGNED - SDG	REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	BORING LOGS II STRUCTURE NO. 061-0093	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
		CHECKED - JMT	REVISED -			327	15-BR	MARION	57	45	
	PLOT SCALE =	DRAWN - DMG	REVISED -			CONTRACT NO. 76A23					
	PLOT DATE = 10/1/2015	CHECKED - SLC	REVISED -			SHEET NO. 16 OF 16 SHEETS					
	ILLINOIS FED. AID PROJECT										